

South Coast Dredging Association

MAREA: Tidal Flows and Sediment Transport Study

Technical Note DDR4323-05



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1. *Introduction*

1.1 BACKGROUND

A Marine Aggregates Regional Environmental Assessment (MAREA) is being undertaken for the South Coast Dredging Association (SCDA) to inform both new marine aggregate dredging licence applications and licence renewal applications off the south coast of England. The study region, shown in Figure 1, includes the coastlines of Dorset and Hampshire between Durlston Head and Hurst Castle; of the Isle of Wight from the Needles to Ryde, and of Hampshire and West Sussex east of Gilkicker Point. However, the tidal harbours of Poole, Christchurch, Portsmouth, Langstone, Chichester, Pagham and Shoreham have been excluded as has the inner part of the Solent.

An important part of the MAREA is to assess the effects of marine aggregate dredging on the physical environment of the study region. At present, such dredging is carried out in numerous areas both to the west and the east of the Isle of Wight by the companies comprising SCDA (CEMEX UK Marine Ltd, Hanson Aggregates Marine Ltd, Kendall Brothers (Portsmouth Ltd), Northwood Fareham Ltd, United Marine Dredging Ltd, Volker Dredging Ltd and Westminster Gravels Ltd). The past, current and planned future aggregate dredging areas within the study region are also shown in Figure 1. In this figure, the dredging areas are coloured differently to show whether they have been already been relinquished, are currently licensed, are proposed new areas for which an extraction licence has already been applied or, finally, are possible future extraction areas which are currently being prospected.

Through Emu Ltd, SCDA appointed HR Wallingford Ltd to carry out an assessment of the effects of past and proposed future aggregate dredging on the physical environment within this study region as a part of this MAREA.

1.2 STUDY OBJECTIVES AND SCOPE

This report considers the effects of changes in seabed bathymetry arising from aggregate dredging, on tidal currents and on the transport of sediment over the seabed that is caused by these currents. Previous studies of planned offshore aggregate dredging around the UK coastline have shown that any predicted changes caused to tidal currents are generally small and restricted to the areas within and surrounding each individual extraction area. Measurable changes in currents have not previously been predicted to occur anywhere further away from any dredging area than the maximum dimension of the area itself.

Since aggregate dredging areas lie well offshore from the coastline, it has generally not been necessary to carry out specific modelling of any changes in tidal currents in Coastal Impact Studies. In some cases, however, aggregate dredging has been proposed in areas close to pipelines or other features of interest on the seabed. The methods used in these cases to examine the effects of a dredging within a single area on tidal flows have been applied in the present study. However, here we examine the regional effects of both past and proposed future dredging in all the licensed and proposed extraction areas in this study region off the South Coast of England.

Any changes to tidal flows will affect the rates and directions of the sediment transport that those currents cause. In previous studies the effects of aggregate dredging on sediment transport have been found to be localised and are only noticeable within, or

very close to the particular dredging area. Certainly, any effects of aggregate dredging on sediment transport rates have not previously been predicted to affect UK coastlines. However, because there are numerous aggregate dredging areas in close proximity within the study region, and there may be plans for other development projects near to these areas, the changes in tidally-induced sediment transport have also been investigated in this report.

It should be remembered, however, that this modelling of tidal flows and sediment transport has been carried out, on a broad scale, for the whole study region. Importantly, this report predicts where there will **not** be changes in these quantities as a result of past and planned future aggregate dredging.

However, this report also indicates where such potentially significant changes may occur. These generally are restricted to within, or to quite small areas close to, the boundaries of the individual extraction areas. If there are specific sensitivities in these areas, for example natural or man-made features that might be affected by changes in flows or sediment transport, these will need to be studied further when an application is made for an extraction licence for the specific dredging area (or areas) closest to the features of interest. These more specific studies will allow the existing seabed and the proposed dredging plans to be represented in greater detail, and hence provide a better assessment of whether the changes in flows or sediment transport rates could have a significant environmental impact.

1.3 CONTENTS OF THE REPORT

The remainder of this report consists of a further four chapters.

Chapter	Contents
2	Chapter 2 describes the numerical modelling procedures used to predict tidal flows, and the sediment transport that they cause, and explains the choices of the model itself, the representations of the seabed bathymetries and the model runs carried out.
3	This chapter presents the results from the modelling and interprets these in the context of the likely changes in the seabed surrounding the dredging areas.
4	Still within the context of possible changes in tidal flows and the associated sediment transport, Chapter 4 discusses the potential interactions between offshore aggregate dredging and other uses of or developments within the study region.
5	Chapter 5 presents the main conclusions, specifically indicating where results from the present study can be used to inform specific licence applications within the study region, or if a further detailed assessment may be required at that time.

2. *Modelling procedure*

2.1 INTRODUCTION

The tidal flow and sediment transport modelling described in this report has been based upon previous projects in which tidal flows and associated sediment mobility and transport in the South Coast region have been investigated.

The mobility and transport of seabed sediments to the east of the Isle of Wight, extending as far as Shoreham, were studied in the first regional study of the UK seabed undertaken to assist in the planning and assessment of marine aggregate extraction. This study was jointly funded by The Crown Estate and SCOPAC (Standing Conference on Problems Associated with the Coast), i.e. the Regional Coastal Group covering the South Coast between Portland Bill and Shoreham. The final report on this study was completed in 1993 (Crown Estate and SCOPAC, 1993). This study concentrated on the offshore part of the seabed, where water depths were at least 10m at lowest tide, and sought to improve the understanding of the natural distribution and movements of sediment. It did not attempt to represent or consider the effects of any human activities such as dredging.

The effects of dredging to the west of the Isle of Wight on the physical environment, including tidal currents and sediment transport, were then investigated in a separate study, carried out by HR Wallingford, the British Geological Survey and the University of Southampton in 1997/98. This study was commissioned by CIRIA (Construction Industry Research and Information Association) and jointly carried out by HR Wallingford, the British Geological Survey and the University of Southampton (CIRIA, 1999). As with the earlier study of the areas east of the Isle of Wight, the main purpose of the study was to describe the natural distribution and movements of sediment, not only over the seabed but also along the shorelines of the Poole and Christchurch Bays. In addition, the effects of aggregate dredging in two hypothetical areas were examined to demonstrate the processes that could be affected and how changes to the natural physical marine environment could be predicted.

These two previous studies provide useful background information on much of the study region shown in Figure 1.

2.2 MODELLING APPROACH

2.2.1 *Regional tidal flow model*

The computational technique chosen to simulate tidal flows in this study is a finite element approach developed by LNH-EDF of France and known as TELEMAC. This software has been used and developed at HR Wallingford for many years in numerous comparable applications. It has the significant benefit of being able to simulate flows over a very large area but also allows a more detailed representation of flows in a sub-area of particular complexity or interest. More details of this modelling technique are provided in Appendix 1 to this report.

The particular flow model that was established and used in this study was based upon an existing HR Wallingford TELEMAC-2D model of the English Channel. This includes the Southern North Sea as far north as Den Helder and Cromer and the English Channel as far west as Lizard Point and Île d'Ouessant. This model has been previously used for a variety of studies (HR Wallingford, 2000, 2002, 2009) and has been shown to

successfully reproduce the general patterns of tidal range and current magnitude throughout the English Channel. The bathymetry for that existing regional model was taken from Admiralty Charts.

In this study, the overall tidal flow model was run by supplying a time history of water levels along the two open boundaries, at the south western (English Channel) boundary and the northern (North Sea) boundary. These water levels were determined from a harmonic analysis using published information from the national BODC database. Calibration and validation of this model has been carried out in previous studies by comparing predicted water levels and currents with observations made by the UK Admiralty at a large number of locations along the coastline, and offshore of them, within the model domain. A fuller description of the comparisons between predicted and measured tidal propagation can be found in Appendix 12 of HR Wallingford et al. (2002).

2.2.2 *Model refinements for the present study*

The very large spatial extent of the existing tidal flow model, i.e. covering all of the English Channel and the southern North Sea, means that it is not possible to represent the seabed morphology at high resolution everywhere. However, when attention needs to be focussed on a particular part of the overall model, as in the present study, it is possible to adjust the mesh spacing locally to provide more detailed and accurate predictions of tidal flows and sediment transport patterns.

The model grid used in the present study was established with much smaller (triangular) finite elements near the existing and proposed dredging areas than further offshore, as can be seen in Figure 2. This refined model mesh allowed the seabed in and around the dredging areas to be well reproduced, and allowed us to adjust the bathymetry to represent the effects of past and proposed future dredging within these areas (see section 2.3). For convenience, Figure 3 shows the dredging areas and the water depths in the study region (before extraction within those areas) at the same scale as Figure 2 so that the relationship between the dredging areas and the finer-resolution parts of the model mesh can be seen.

Defining changes in seabed levels using this finer-spaced mesh means that, in turn, the extent and magnitude of the likely changes in tidal flows and sediment transport in and around each of the dredging areas can be predicted in detail, while still allowing a regional overview of the combined effect of all such dredging in the study region.

It should be noted, however, that there was no scope in the present study to validate predicted tidal currents in and around the dredging areas following the refinements to the existing model, for example by using measurements. This may be advisable if there is a concern that the changes in flows identified in this regional and broad-scale modelling study could have an adverse effect on the environment or other features of interest, requiring more detailed consideration at a later date.

2.2.3 *Sediment transport*

Any changes in tidal currents predicted by the TELEMAC flow model as a result of past or planned future aggregate dredging will alter the sediment transport that the tides cause. The extent and magnitude of such changes to transport, however, will also depend on the type of sediment present, i.e. its size and weight, on the morphology of the seabed, i.e. its slope and roughness, and on the particular tidal conditions. Fine and

medium-sized sand, for example, is much more likely to be disturbed and transported by tidal currents than coarser sand or gravel. The sediments on the surface of the seabed in the study region vary very considerably from area to area, and it is impractical for an REA to examine the transport of all of the many different types of sediment within the study region.

Sediment transport in this area is also much weaker when tidal ranges are smaller, during neap tides than during spring tides. The effects of winds and variations in atmospheric pressure in the English Channel also affect tidal levels and currents, particularly when storm surges occur. Again however, it was not practicable to consider a wide range of different tidal conditions and their effects on sediment transport in this study.

However, it was not necessary to consider all of the many factors affecting sediment transport in this study, where the emphasis was on determining the likely extent and magnitude of any changes caused by aggregate dredging. The approach chosen in this study, therefore, seeks to identify where there may be noticeable and frequent changes in sediment transport as a result of aggregate extraction. In this way, attention can be drawn to those areas where such dredging may have a potentially significant effect on the environment and, conversely, identify other areas where no changes will be experienced.

In order to interpret possible changes to sediment transport in the context of the potential effects on either the natural environment or on man-made feature or activities, it was decided to err on the side of caution in the choice of the tidal conditions and sediment grain sizes studied. If in some areas, for example close to a coastline, the changes in predicted sediment transport under these assumptions were zero, or too small to be a concern, then it can be concluded that this would also be the case when tidal ranges were smaller or if the sediment particles in those areas were larger and hence less affected by changes in tidal current speeds. For this reason, we have chosen to consider the transport of medium sized sand (with a median grain diameter of 0.3mm). Over most of the part of the study region where aggregate dredging takes place, the seabed sediments tend to have a proportion of gravel and will be much less mobile than this medium sized sand. The effects of dredging on the rates of transport of that coarser sediment will be, in reality, smaller than predicted by our numerical modelling.

Sand transport was simulated in this study using the HR Wallingford SANDFLOW model, which is a non-equilibrium finite element sediment transport model that simulates the total load (suspended and bedload), with input flows from TELEMAC. Further details on this sediment modelling technique can be found in Appendix 2.

While this model has the capacity to represent the additional effects of wave action on sand transport, we have not attempted to do this in the present study. While waves can affect the rates of sediment transport, the predictions of the spatial extent and pattern of changes in transport caused by aggregate dredging will not be very different if undertaken with or without the effects of wave stirring. Although SANDFLOW has the capability of masking discrete areas of the seabed with deposits of the grain size for simulation (with other areas devoid of sediment) all model simulations were performed assuming an abundant supply of material over the whole study region. This approach is preferred because it results in model predictions that are not dependent on this initial starting condition. However, this does require care to be taken when interpreting the model results.

The SANDFLOW model can output, instantaneously, the suspended sediment concentration and sediment flux throughout the period simulated (typically an individual tidal cycle). However, for the present study it was considered more useful to provide information on the net sediment transport over a tidal cycle to focus on the longer-term transport rates and pathways. It is this net transport that has been analysed in detail in this report. This approach is appropriate when considering the possibility of aggregate dredging leading to deposition on or erosion of sediments from the seabed surrounding the extraction areas. In general, the possibility of changes in the seabed morphology is usually more of a concern than changes in sediment transport rates themselves. Any increases in net transport rates suggest possible erosion (scour) of the seabed while reductions may lead to deposition (siltation) of sediment.

2.3 SPECIFYING SEABED BATHYMETRIES IN THE STUDY REGION

2.3.1 Introduction

Aggregate, i.e. sand and gravel, has been extracted from the seabed offshore from the central South Coast of England for many years, and the presently licensed dredging areas are shown in Figure 1. When the MAREA is complete, the SCDA companies will apply individually to renew their extraction licences, several of which expire in the next few years. In addition, some companies are considering making applications for extraction licences for new areas within the study region.

The purpose of this report is to investigate how past and proposed future aggregate dredging in the study region, on its own, has or might alter tidal flows and the associated transport of sediment over the seabed. In order to concentrate on just this cause of change, representations of seabed levels in the region were produced which differed from each other only in those areas where aggregate dredging has taken place or is proposed in the future.

To assess the impact of aggregate dredging on tidal flows, three representations of the bathymetry within the study region were produced.

- A “baseline” bathymetry, in which the seabed levels in each existing or relinquished dredging area represent the situation before any dredging was started. We refer to this as the “pre-dredged” bathymetry;
- A “present day” bathymetry in which seabed levels in each existing or relinquished dredging area have been established using the latest survey of those areas. The differences in bed levels between this and the “pre-dredged” bathymetry will very largely be the result of past dredging in existing or relinquished licence areas; and
- A “post-dredging” bathymetry in which future seabed levels in each existing and proposed new dredging area have been defined. These have been predicted by combining the present-day bed levels and the present plans of SCDA members for extraction in the medium term future.

For the assessment of changes in tidal flows and sediment transport presented in this report, we have examined the effects of all past and proposed future dredging together. As a result, only the first and third of these bathymetries were used in the modelling of tidal flows and associated sediment transport. Despite this, it is helpful to describe how all three of the bathymetric representations were produced, and this is the subject of the following two sub-sections.

2.3.2 *Representing the past and present-day bathymetries*

Starting first with the representation of the past and present-day bathymetries, it is important to note that the sole purpose of this report is to investigate how past and proposed future aggregate dredging might alter tidal flow conditions, and the associated sediment transport, in the study region. In order to concentrate on just this cause of change, therefore, these two bathymetric representations were produced so that they differed from each other only in those areas where aggregate dredging has taken place. “Present day” bed levels in existing dredging areas were obtained from the aggregate dredging companies. They also provided information on bed levels in those areas before dredging started and the “pre-dredging” bed levels were produced using those older surveys or information on patterns and amounts of past dredging, or a combination of the two.

For the remainder of the study region, i.e. outside the past and present aggregate dredging areas, the bed levels in the study region were assumed to be the same for both bathymetries and were taken from the most recent digital bathymetric data set available from the UK Hydrographic Office (via Seazone Solutions Ltd).

Figure 4 shows the changes in depths west of the Isle of Wight between the present-day and the pre-dredging bathymetry, and Figure 6 shows the same differences in depths within the dredging areas to the east of the Isle of Wight. The differences in bed levels in these two bathymetries are very largely the result of past dredging, but in some cases, small depth changes are found in parts of the licensed areas where no dredging has taken place. This is either a result of natural sediment transport processes altering the seabed, for example the movement of sand waves or sandbanks or because of slight differences in the interpolation of often coarsely-spaced depth soundings in the older survey data that has been used.

It is important to realise that neither of these bathymetric data sets can be ascribed to any particular year. In this large study region, the digital data on seabed levels available from the Hydrographic Office is a collage of information from many different surveys carried out over the last 10-15 years (and possibly even longer ago in some areas). Similarly, the information provided by SCDA members is from surveys that were carried out in different years in different dredging areas, reflecting the fact that dredging was started in the various areas at different times.

In addition, even if it was possible to obtain a complete and detailed survey of the seabed over the whole study region at some date before dredging started and another very recent similar survey of present-day bed levels, many of the changes in depths between these dates would be caused by natural processes rather than by aggregate dredging. The numerical predictions of changes in tidal currents between these two dates would not then be able to discriminate between the effects of aggregate dredging and those caused by natural changes, such as the movement of sandbanks, or by other human activity, for example dredging navigation channels and depositing sediments arising from such dredging in designated areas on the seabed.

2.3.3 *Representing future dredging*

For future dredging, whether from existing or possible new areas, the SCDA members have defined water depth increases that might be brought about from proposed dredging to the year 2030 (approximately). In general, these changes have been based on their accurate information on the depths and spatial extent of the sand and gravel deposits in each area. In some areas, the proposed depth changes would remove all virtually of this

sediment. Figures 5 and 7 show the depth changes that would be caused by these plans for further dredging in this sub-region (i.e. the post-dredging minus the present-day bathymetry). Within any individual area, the future depth changes shown in Figures 5 and 7 are considered realistic in terms of the known resources of sand and gravel, and are achievable within the expected lifetime of any new extraction licences. This applies for both proposed new extraction areas and for existing areas from which further dredging is planned

However, the actual amounts dredged in each individual area by about 2030 will, in reality, depend firstly on the scale of applications made for licence extensions (for existing areas) or for new areas, secondly on the conditions that may be placed on permissions to dredge in these areas and thirdly on the proportion of the permitted extraction that actually takes place during the lifetime of any licence (typically up to 15 years). Some of these licences may be granted within a few years from now while other licence applications may take much longer to prepare and then be determined.

Achieving all the depth changes shown in Figures 5 and 7 by about 2030 would result in a much larger volume of aggregate being extracted than is expected to be achieved in reality, based on past rates of dredging in this region. In this particular study, therefore, the predicted changes in tidal flows or sediment transport rates in and around any individual dredging area could occur. However, this would mean that changes around another area, or areas, might be over estimated.

Finally, details of all the information received by HR Wallingford on changes in bed levels (past and proposed) in the various dredging areas are summarised in Appendix 3. The three bathymetric data sets were then compared and checked to ensure that in each area the changes in bed levels between pre-dredging and present day, and between present-day and post-dredging were, respectively, a good representation of the past and the presently-planned future dredging.

2.4 CHOICE OF INPUT CONDITIONS

The final step in the establishment of the modelling process was to choose the conditions to be input to the flow and sediment transport model. These choices are explained below.

2.4.1 *Selection of tidal range*

As mentioned in Section 2.2, the main objective of this sediment transport modelling is to identify whether aggregate dredging is likely to alter sediment transport patterns on a regular basis, and to provide guidance on which areas of the seabed might experience deposition of sediment or, in contrast, where erosion might take place.

For this purpose, we have therefore chosen to consider a single tidal range equivalent to that of a mean Spring tide of 4m at Chichester. This same tidal range was used for much of the flow and sediment transport modelling undertaken for the South Coast Seabed Mobility Study (Crown Estate and SCOPAC, 1993).

2.4.2 *Selection of seabed roughness length values*

Because of the wide range of sediment types on the seabed in the study region (see SCOPAC and Crown Estate (1993) and CIRIA (1999)), the hydraulic roughness of the bed, which influences both tidal propagation and current speeds, varies considerably.

For the broad-scale modelling described in this report, we have used a representative value for the Nikuradse grain roughness, k_s , of 0.1m which we feel is representative for this study region.

2.4.3 *Selection of sediment grain size*

In the present study, interest needs to be focussed on the seabed in and close to the various offshore aggregate dredging areas, which are generally on parts of the seabed where the surface sediments are gravel or sandy gravel. To err on the side of caution, in this study we have chosen to predict the transport of medium-sized sand, with a median grain size of 0.3mm. Over much of the study region, the seabed surface is exposed bedrock or covered with a nearly immobile mixture of sand and gravel. Both the actual rates of sediment transport, and any changes in these rates caused by aggregate dredging, will in reality be smaller than indicated in this regional study, which therefore errs on the side of caution.

2.4.4 *Choice of seabed bathymetries for use in the modelling*

In modelling that was carried out in an earlier stage of the present MAREA (HR Wallingford, 2010a), it was decided to present results on the effects on **waves** of past dredging and then of past and all future dredging combined, which showed substantially greater changes than those caused by the aggregate dredging already carried out. The subsequent interpretation of those modelling results, and the recommendations made in respect of further studies, were very largely based on the results obtained from modelling the combined effects of past and planned future dredging.

In view of this, in the modelling of the effects of aggregate dredging on **tidal flows**, and associated sediment transport, it was decided only to compare results obtained using the pre-dredging (i.e. baseline) and the post-dredging bathymetries. The changes predicted by this approach will be larger and more widespread than if only past dredging was represented. More importantly, however, future studies and any decisions made about future licence applications will be based on the results obtained by considering the combined past and future dredging in the study region. By taking this approach and bearing in mind the over-estimation of the total future dredging in this region, this report errs on the side of caution.

In summary, therefore, the tidal flow and sediment transport models were run for first the pre-dredging bathymetry and then for the post-dredging bathymetry. The changes in the results were then compared to assess the potential cumulative effects of all past and proposed aggregate extraction.

3. *Discussion of results*

3.1 PRESENTATION OF RESULTS

The purpose of the study is to show how aggregate dredging might have altered, or may in future alter tidal current speeds and net sediment transport rates over the whole study region. The main results from the computational modelling are presented here to provide an overview of the spatial extent and magnitude of predicted changes both within and outside the numerous dredging areas.

There is no simple way of assessing whether or not such changes may be significant in the sense of having the potential to provide some environmental benefit or cause harm. This judgement can only be made with detailed knowledge of the environmental sensitivities of the areas within which changes are predicted to occur.

For example, the Marine Life Information Network (MarLIN) points out that “The sensitivity of a species (or community) is an estimate of its intolerance to damage from an external factor and is determined by its biological and physical characteristics. Sensitivity must be estimated (assessed) in response to a change in a specific environmental factor and to the magnitude, duration, or frequency of that change.” In this specific biological context, MarLIN presents the following characterisation of Tidal Strength:

Very strong	>3 m/s
Strong	1.5-3 m/s
Moderately strong	0.5-1.5 m/s
Weak	<0.5 m/s
Very weak	Negligible

(see <http://www.marlin.ac.uk/glossaries/waterflowrate.php>)

MarLIN point out that many species and biotopes occur under a range of water flow conditions. A prolonged change in current speeds of two categories is more likely to affect a range of species than if current speeds only move to an adjacent category (see ‘Changes in Water Flow’ <http://www.marlin.ac.uk/sensitivitybenchmarks.php>).

In interpreting the results presented in this report, therefore, there could well be a potential change in marine plant and animal communities if tidal speeds were to change by two categories or more, e.g. from just under 1.5m/s to over 3m/s or from just over 1.5m/s to less than 0.5m/s. This would correspond to current speed changing by more than 50%. If the predicted changes were only a few percent, however, the MarLIN guidance suggests that the effects on marine life would not be particularly concerning.

However, there may be other features of interest, for example wrecks or pipelines, close to aggregate dredging areas which might be more sensitive to changes in current speeds or in net sediment transport rates. In presenting such changes, it should be remembered that any numerical modelling of tidal flows, and especially of sediment transport, inevitably has a degree of uncertainty. This means that it is not possible to be confident about any very small changes that are predicted to occur as a result of aggregate dredging. A judgement therefore has to be made about the potential significance of predicted small changes in current speeds.

In this report, we have not shown changes where the modelling indicates that aggregate dredging would only alter peak current speeds by 2% or less. This type of change is comparable to the variations in peak tidal current speeds on two days when the same tidal range occurs. Similar comments apply to the predictions of net sediment transport rates, as discussed in Section 3.3.

Even if changes greater than this are shown within an area of particular interest, the predictions made in this regional study should be treated as cautionary rather than precise. Any potentially significant changes may need to be assessed in more detail during the Environmental Impact Assessment studies for the specific dredging area, or areas, closest to the particular features of interest. Any small changes in current speeds

that are determined as being ‘not significant’ at this regional scale would not be expected to require further modelling during the licence-specific EIA stage.

As pointed out above, in other parts of this MAREA it was found that changes in tidal currents due to past dredging were predicted to be small compared to those likely to be caused by planned future dredging. To err on the side of caution in this particular modelling exercise, we have therefore studied the effects of both past and future dredging together. To do this, we have run the tidal flow and sediment transport models for the pre-dredged bathymetry first and then the post-dredged bathymetry, and compared the results from these two simulations. The changes predicted by this approach will be greater than if we had only considered either the past dredging or the planned future dredging in isolation.

Even with the restricted number of model runs carried out, i.e. for just two bathymetries, a single mean spring tide and a single sediment size, the volume of computational model results that could be presented is very substantial. However, to interpret the results in the specific context of assessing the greatest changes that aggregate dredging might cause, it is possible to use a much smaller number of those results. For example, at about the time of high and low tide, tidal currents and sediment transport rates are much smaller than they are roughly three hours before or after those times. It is likely for any practical purpose, for example the consideration of scour around seabed structures, that it is the changes in the fastest current speeds that will be of most concern. Section 3.2 below presents and interprets the changes in current speeds that would cumulatively result from all the past and proposed future dredging in the study region.

Following the consideration of changes in the speeds of tidal currents, if and when there are any concerns about the potential for long-term changes in the seabed morphology outside the dredging areas, changes in the net sediment transport rate over the tidal cycle are of much greater interest than the transport rate at any particular stage during that cycle. We consider this particular aspect of changes caused by dredging in Section 3.3.

3.2 EFFECTS OF CHANGES IN BATHYMETRY DUE TO AGGREGATE DREDGING ON TIDAL FLOWS

The TELEMAC flow model runs carried out in this study allow an instantaneous snapshot of the current speeds and directions at any location, and indeed over a much larger region offshore from the South Coast, at any stage of the tidal cycle. As examples to illustrate the strength of currents, Figures 8 and 9 show the predicted flows at the peak of the flood tide and ebb tide respectively for the present-day bathymetry. Equivalent figures to these for the pre-dredging or post-dredging bathymetries would be almost identical, given the modest changes in current speeds caused by aggregate dredging, as explained below.

These figures (plotted on a regular model grid for ease of viewing) show that the peak currents across the seabed in the vicinity of the dredging areas flow roughly to the east and north-east on the flood tide, and roughly west and south-west during the ebb tide. The spatial variations in the current speeds in the area north-east of the Isle of Wight reflect the presence of large sandbanks (shown in Figure 1). The current speeds are faster off the southern tip of the Isle of Wight, compared to the area offshore of Swanage, Selsey Bill and Brighton. Over much of the area of greatest interest, the flows at the peak of the flood tide are greater than during the ebb tide.

Producing similar plots for the pre-dredging and post-dredging bathymetries is entirely possible, but identifying the changes between these when presented in the same format as Figures 8 and 9 is very difficult. To draw particular attention to the changes that past and proposed future dredging would cause, therefore, a different presentation of the model results has been used. Figures 10 and 11 summarise the **differences** in current speeds at the peak of the flood tide and ebb tide as predicted for the post-dredging and pre-dredging bathymetries. The changes due to past dredging alone, in particular, would be much smaller than those presented in these two figures.

These comparisons of the pre-dredging and post-dredging situations show that tidal currents are most strongly increased at the western and eastern ends of the dredging areas. This is because the increased water depth attracts a greater tidal discharge through those areas. As this increased discharge enters and leaves the dredged areas, it causes faster current speeds over the un-dredged areas of the seabed just upstream and downstream. In some areas, flows alongside the dredging areas tend to reduce. Within the dredging areas themselves, the increase in water depths and discharge often result in little change in the depth-averaged current speeds, although minor increases or decreases can occur depending on the bathymetry variations in each area, reflecting for example different depths of dredging within each area.

It should be borne in mind that changes in tidal currents in and around the dredging areas in this study region have been predicted assuming that all the proposed future dredging in each area was carried out. This is a very conservative assumption. If the actual amounts of extraction within these areas are smaller, then the predicted extent and magnitude of changes in the tidal flows in and close to these areas will be reduced.

In overview, the results shown in Figures 10 and 11 demonstrate that predicted changes in peak current speed of greater than 5% are restricted to the dredging areas themselves or extend, in most cases, no more than a few hundred metres beyond the limits of these areas. The parts of the seabed over which the peak current speeds are predicted to change by between 2% and 5% are also shown on these figures. As noted previously, when considering possible effects on marine plants or animals, it seems most unlikely that changes in peak current speed of less than 5% would have any damaging effect, but this cannot be entirely ruled out, since any such judgment would require knowledge of the sensitivity of, for example, the environmental receptors within these areas.

In the light of these results, it seems unlikely that changes in tidal currents will result in any noticeable changes to the environment except, perhaps, where there are either existing features of interest or existing/ planned developments that are very close to the boundaries of the various dredging areas. Where this is the case, changes in tidal currents, and the effects of these changes, might need to be studied in greater detail at the time of applying for an extraction licence.

Where no changes in current speed are shown in Figures 10 and 11, then this indicates that the model is predicting the same situation before and after dredging to within the expected range of accuracy of the computations made; in these areas, shown in white, it can be assumed that **no** changes in tidal currents will be experienced as a result of all past and proposed future dredging.

More specifically, based on the consideration of both past and future dredging in this study, there is no need for any concern about the effects of past or proposed future dredging on tidal flows along or close inshore to any part of the coastline within the study region.

3.3 EFFECTS OF AGGREGATE DREDGING ON SEDIMENT TRANSPORT

In those areas where peak tidal currents are predicted to be altered by aggregate dredging, as summarised in Figures 10 and 11, this will result in some change to sediment transport rates at those times. The net sediment transport rate over a tidal cycle, however, does not depend solely on the current speeds at the peak of the ebb and flood tides; rather it is the sum of all the instantaneous sediment transport rates at every stage of that cycle.

It was decided to build upon the modelling of tidal currents described above and predict the net sediment transport rates over a spring tide cycle for both the pre-dredging and post-dredging bathymetries. However, the results of this sediment transport modelling need to be interpreted with some care. In this regional study, the main interest is not in detailed estimates of how much the sediment transport rates might change as a result of dredging. Any numerical modelling of sediment transport rates over the SCDA study region would, in any event, be subject to considerable error because of the considerable variations in tidal ranges and sediment grain sizes, which cannot be adequately investigated within the constraints of this regional study.

What is more important, and practical, is to identify those parts of the study region where potentially significant changes in sediment transport patterns and rate changes **might** occur. This in turn would help to identify those areas where changes in the seabed morphology may be caused as a result of aggregate dredging. By considering medium-sized sand and a larger tidal range than average, the modelling carried out will over-estimate the areas within which the natural sediment transport regime might be affected by past and proposed future aggregate dredging. As for other aspects of this study, it was considered that the effects of past dredging on sediment transport were likely to be smaller and less of a concern than proposed future dredging. By considering the effects of both past and all future dredging, our modelling has erred on the side of caution.

Our modelling has predicted the net sediment transport over a spring tide of medium-sized sand (with a median grain size of 0.3mm) over the whole SCDA region, first for the pre-dredging bathymetry and then again for the post-dredging bathymetry. These results showed a very wide range of net sediment transport rates, ranging from virtually zero to several thousand cubic metres per metre width of transport path each spring tide. Because of this, it is not possible to present changes in sediment transport rates meaningfully using the same methods as used to produce Figures 10 and 11, where a simple percentage increase or decrease is shown. Figure 12 therefore shows where natural, i.e. pre-dredging, transport rates are predicted to change as a result of past and proposed future dredging by more than 100 kg per metre width of seabed per tide. This is the smallest change in sediment transport rates that we feel is safe to identify using our modelling method.

As with many other aspects of the physical environment, Figure 12 shows that noticeable changes to sediment transport rates as a result of all past and proposed future dredging are very largely restricted to the dredging areas themselves, or to a localised area around their boundaries. Note that this figure does not take into account whether there is sand sized sediment available to be transported at a given locality. In many parts of the dredging areas, the seabed sediment is sandy gravel, which is naturally immobile. In these areas, the existing sediment transport rates, and the changes in these that would be brought about by dredging would be smaller than shown in Figure 12.

Changes within the dredging areas themselves are varied, since the increase in tidal flow caused by the bed lowering may mean that the discharge increases as well as the water depth. As a result, it is not easy to tell in advance whether current speeds and sediment transport rates will increase or decrease. However, based on experiences to date, such changes to the hydraulic regime of these areas are usually of little concern in comparison to the changes to the environment within those areas caused by the dredging process itself.

If there are existing or planned developments or features of interest within or close to the boundaries of one of the extraction areas, then there might be a need to carry out more detailed investigations of how the seabed may alter at the time of preparing a formal extraction licence application. At that stage, a more detailed assessment of the amounts and type of seabed sediments can be taken into account, along with more detailed bathymetry than that used in this regional study, to assess changes to the sediment transport regime. This would then allow a more detailed assessment of whether there would be siltation or erosion at locations of interest close to any dredging area.

Finally, it should be noted that this study only predicted the changes in tidal currents and sediment transport after all dredging has been completed. The transport of sediment disturbed by dredging operations themselves, and the effects of this on the seabed, is considered in a companion report (HR Wallingford, 2010b).

4. *Aggregate dredging and in-combination effects*

Aggregate dredging is only one of many human activities that affect the physical environment of the shorelines and offshore waters of the central South Coast region of England. Other activities include:

- Some types of commercial fishing that cause disturbance of the seabed and its sediments;
- Dredging carried out for navigational purposes and the associated disposal of the dredged material, which not only alters the water depths but also potentially alters waves, tidal currents and the movement of sediments over the seabed;
- The construction of breakwaters, seawalls and groynes along the coastline of the region, which has often led to very significant changes in waves, currents and sediment transport rates locally, leading to changes in the nearshore seabed; and
- The seabed in the study region is crossed by cables, and outfalls etc, and these both affect and are affected by waves, tides and sediment transport processes.

For some of these activities and developments, comprehensive studies are routinely undertaken to assess the impacts they may have on the environment, including the physical environment, as part of the process by which necessary consents are obtained. For others, such environmental studies are not required or are more limited in scope. In general, however, all such assessments concentrate on an individual project and its environmental effects rather than considering the possible “in combination” effects of a specific project with others nearby, even if these are of a similar nature.

Over the past 20 years, the marine aggregates industry has included the cumulative effects of all currently licensed and proposed new dredging areas when assessing the effects on the coastline of planned future extraction. This present study is an extension

of this well established practice. It provides an indication of how aggregate dredging in multiple areas, often very close to one another, could affect tidal currents, and hence in the associated sediment transport of medium-grained sand, anywhere within this study region. Where future developments or uses of the sea close to any of the dredging areas are planned, this study helps quantify both the present-day and the future tidal flows and sediment transport rates in those areas and can thus contribute to the design and the assessment of the effects of those future developments.

This regional study of tidal flow and sediment transport has not tried to quantify any potential effects of aggregate dredging that might affect the marine physical environment “in combination” with effects from other human activities, e.g. due to the deepening of a navigation channel or the construction of coastal defence schemes. This would be a very complicated task, not least since it is not easy, and sometimes not possible, to obtain details of any other planned developments or operations within the study region.

Overall, it is reasonable to conclude from this study that any in-combination effects on tidal currents and sediment transport involving aggregate dredging are unlikely unless other projects are located close to boundaries of those dredging areas. Consequently it is not considered possible that there would be any such in-combination effects with projects along the shorelines in the study region, for example the construction or maintenance of coastal defences.

Some of the specific activities and developments within the study region with a potential to combine with aggregate dredging to produce an effect on the environment are considered within the in-combination impacts section of the overall Marine Aggregate Regional Environmental Assessment (MAREA) to which this study contributes.

However, any judgement about the magnitude of such interactions would require specific information and more detailed modelling than has been possible in this regional study. Potentially significant interactions that are identified will therefore need to be considered in more detail in the Environmental Impact Assessment (EIA) carried out for the extraction licence application for a specific dredging area.

5. *Conclusions*

This modelling study has assessed the changes in tidal currents, and in associated sediment transport rates of sand, that would result from both past and proposed future aggregate dredging in the SCDA study region. The main aim of this modelling has been to identify the likely spatial extent of potentially noticeable changes in currents and sediment transport rates.

The modelling has erred on the side of caution, by considering only a spring tide, when currents are stronger than average, and by assuming the surface sediment is medium-sized sand, when in reality much of the seabed is covered by coarser sand and gravel. Because of this, the spatial extent and magnitude of the predicted changes in both current speeds and net sediment transport rates are greater than will occur in reality. In addition, the total amount of dredging within this study region is considerably larger than is likely to take place before about 2030.

Even by adopting this precautionary approach, for many of the existing licensed dredging areas in the study region, predicted changes in current speeds of greater than

5% do not extend outside the boundaries of the dredging areas themselves. Because of this, any changes in the morphology of the seabed in the study region following aggregate extraction will, at worst, be limited to very restricted parts of the seabed adjacent to the existing or proposed dredged areas boundaries.

For some areas, where plans for future dredging involve lowering bed levels by several metres, modest changes in peak current speeds, i.e. 5% to 10%, and in net sediment transport rates, do extend beyond the licence boundaries, in places by several hundred metres. While these changes do not seem likely, for example, to have a major impact on marine plants or animals within these limited areas, the present study cannot assess the significance of any changes to the environment. This will depend upon the locations and sensitivities of any features of interest within these areas.

In summary, therefore, this regional scale modelling exercise is sufficient to assess the potential effects on tidal currents or sediment transport of both past and proposed future extraction from most of the dredging areas in the study region. When licence applications are submitted for an individual area, any further more detailed assessments of such effects would only be warranted for those areas where this present regional study has indicated changes extending outside the boundaries of that area, and where there are existing or proposed features of interest in the area where such changes have been predicted. Examples of such features could include pipelines, offshore wind turbines and historic wrecks, and possibly natural features of the seabed, for example assemblages of marine plants and animals.

Where such further studies are considered appropriate, the present study will provide a good starting point from which more detailed and localised assessments of changes in tidal currents and sediment transport can be carried out.

Finally, even by considering the cumulative effects of all dredging areas in the study region, and assuming that the planned maximum amounts of sand and gravel are removed in the next 15 years or so, there is no evidence from this modelling of any changes in tidal currents, or in sediment transport rates, along or close to any coastline.

6. *References*

Crown Estate and SCOPAC (1993). “South Coast Seabed Mobility Study – East of the Isle of Wight”. Also published as HR Wallingford report, EX2795, July 1993.

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Figures

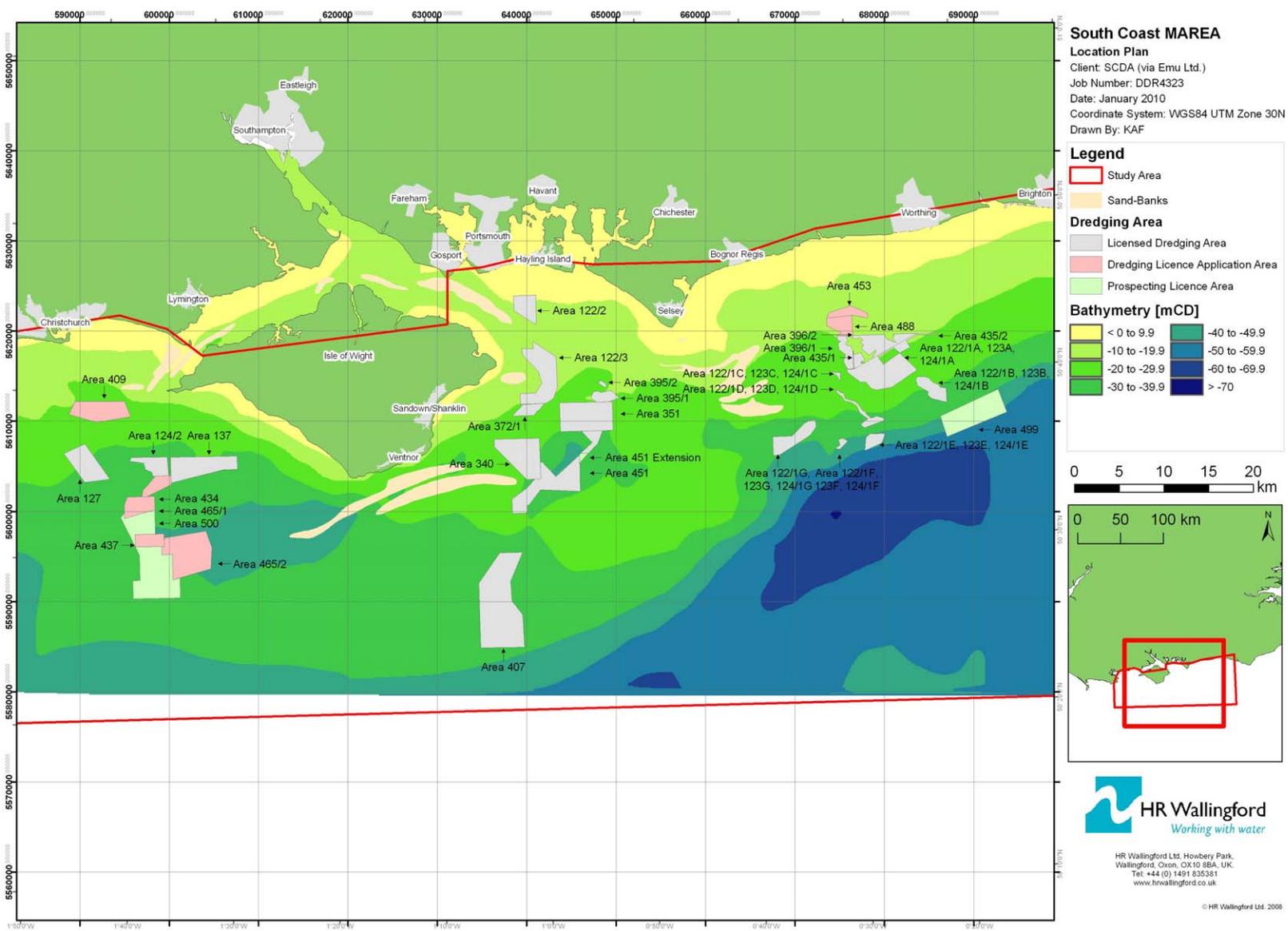


Figure 1 South Coast region boundaries, dredging areas and bathymetry

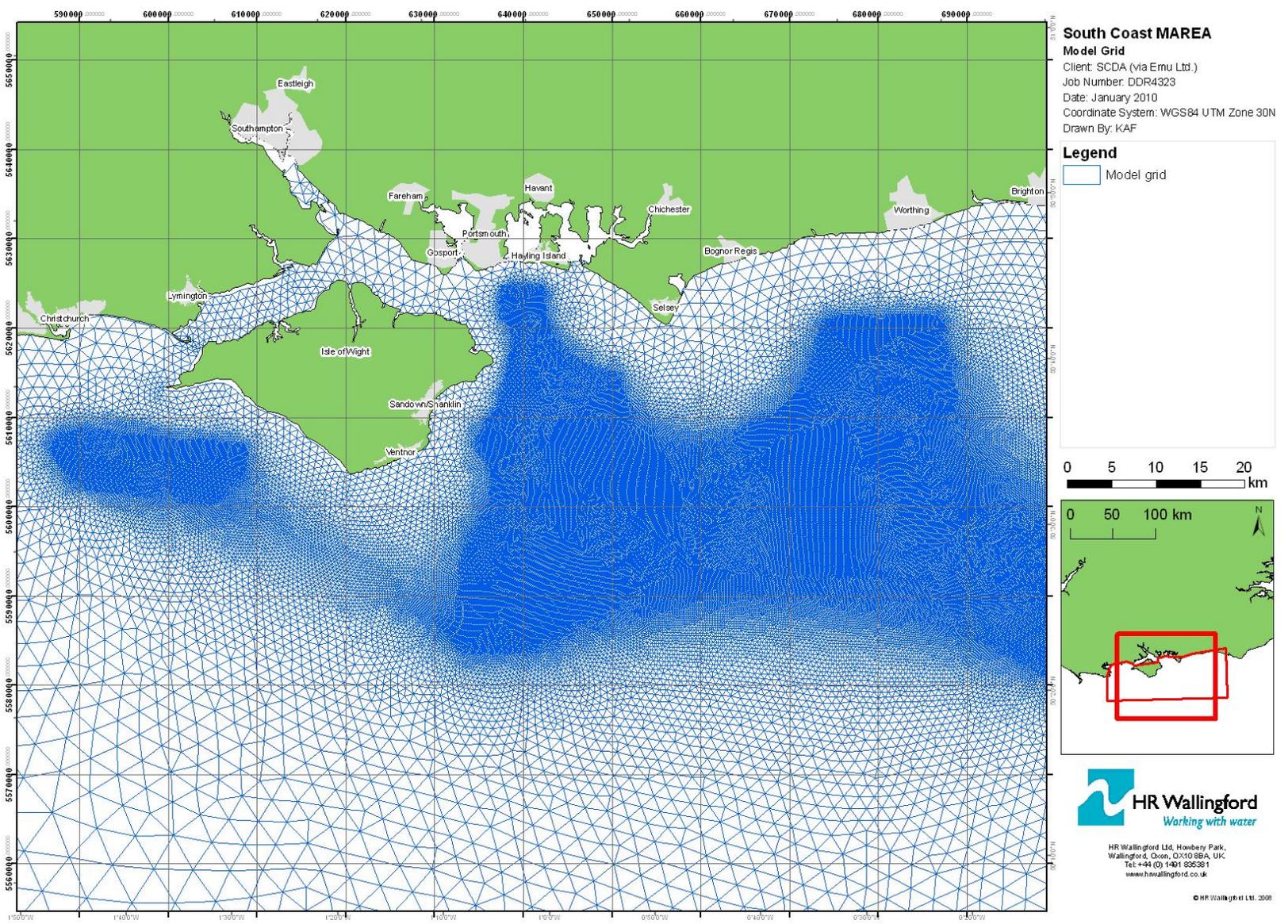


Figure 2 South Coast Region – Tidal flow model grid used in this study

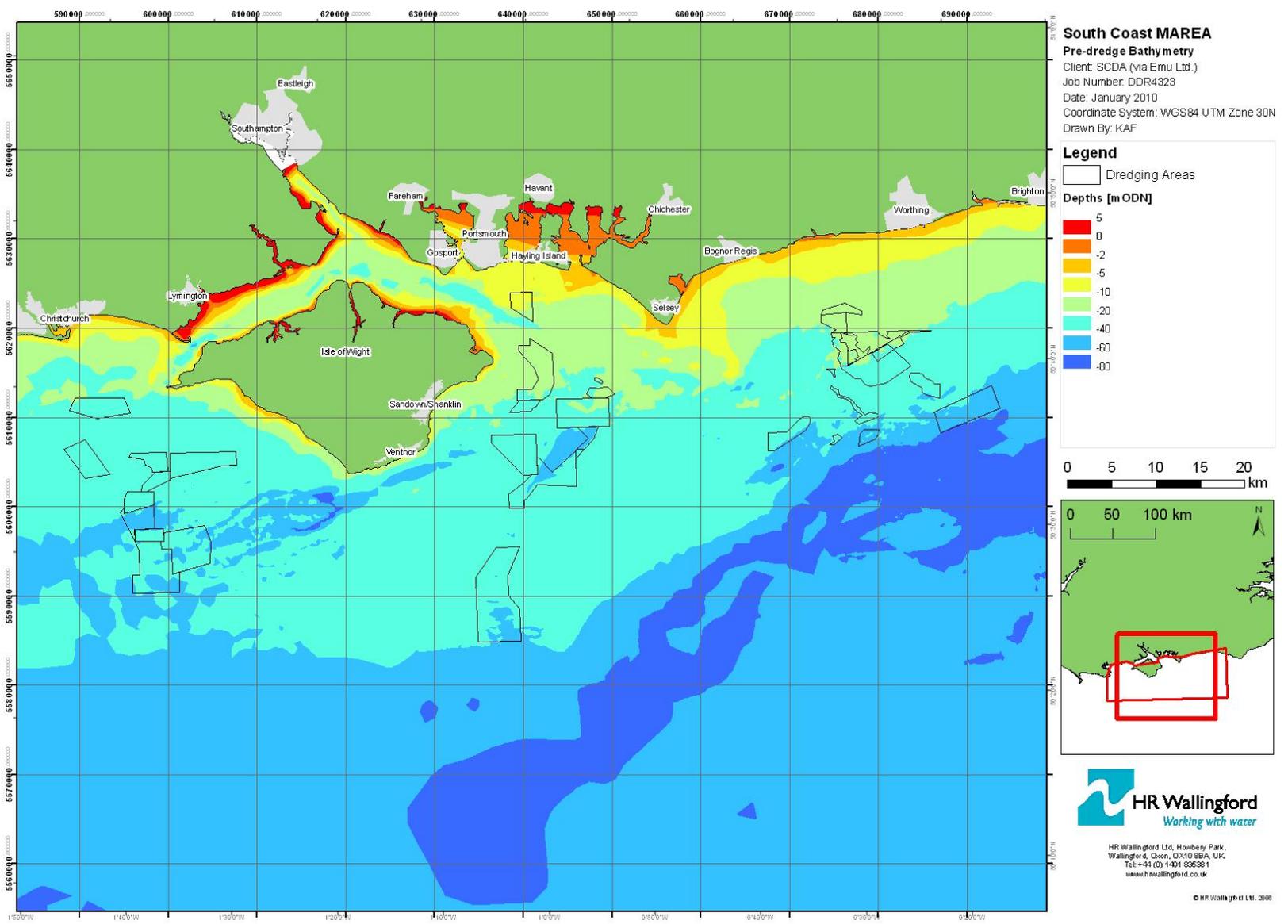


Figure 3 South Coast Region – pre-dredge bathymetry

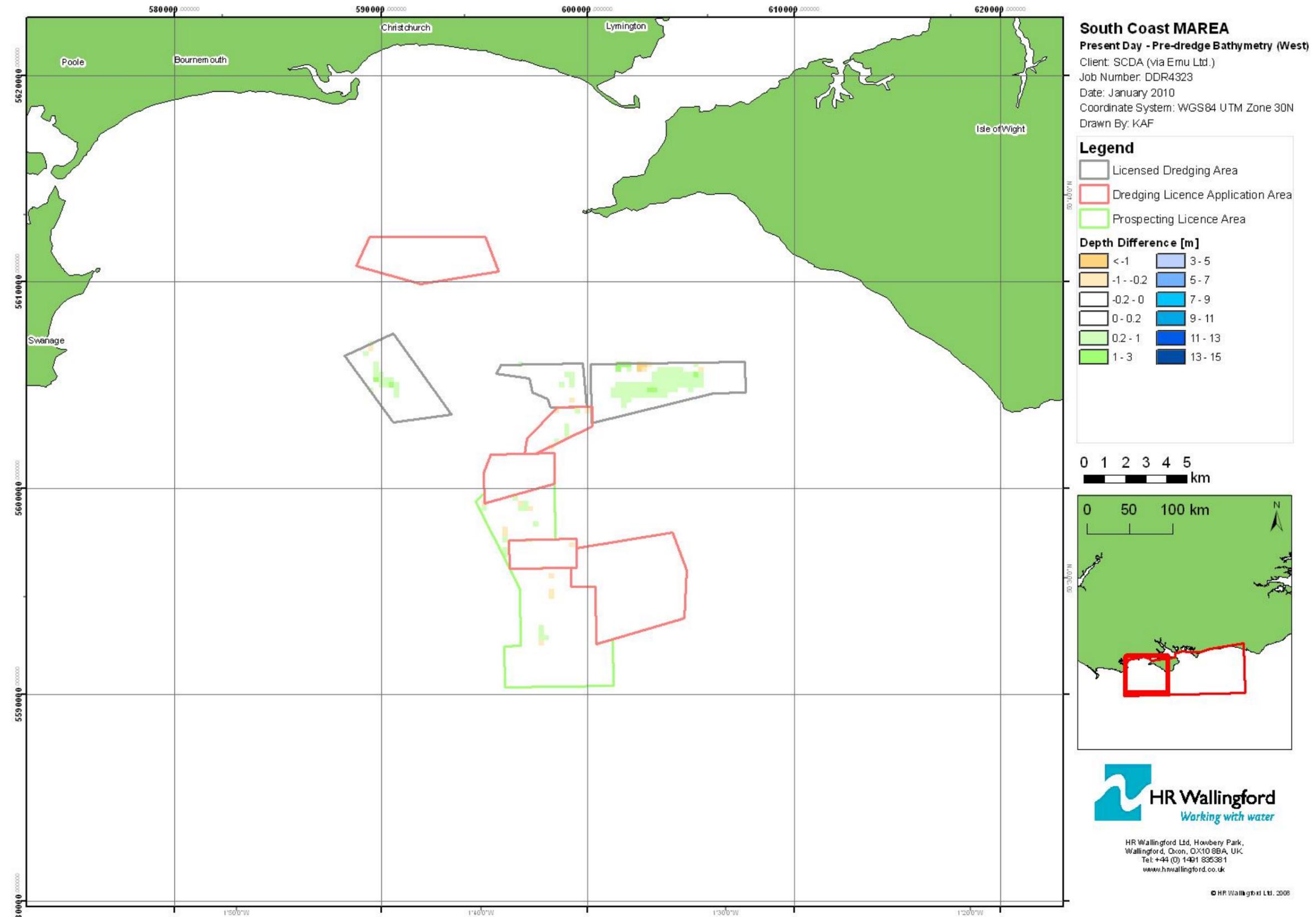


Figure 4 Past depth changes within existing licensed dredging areas west of the Isle of Wight

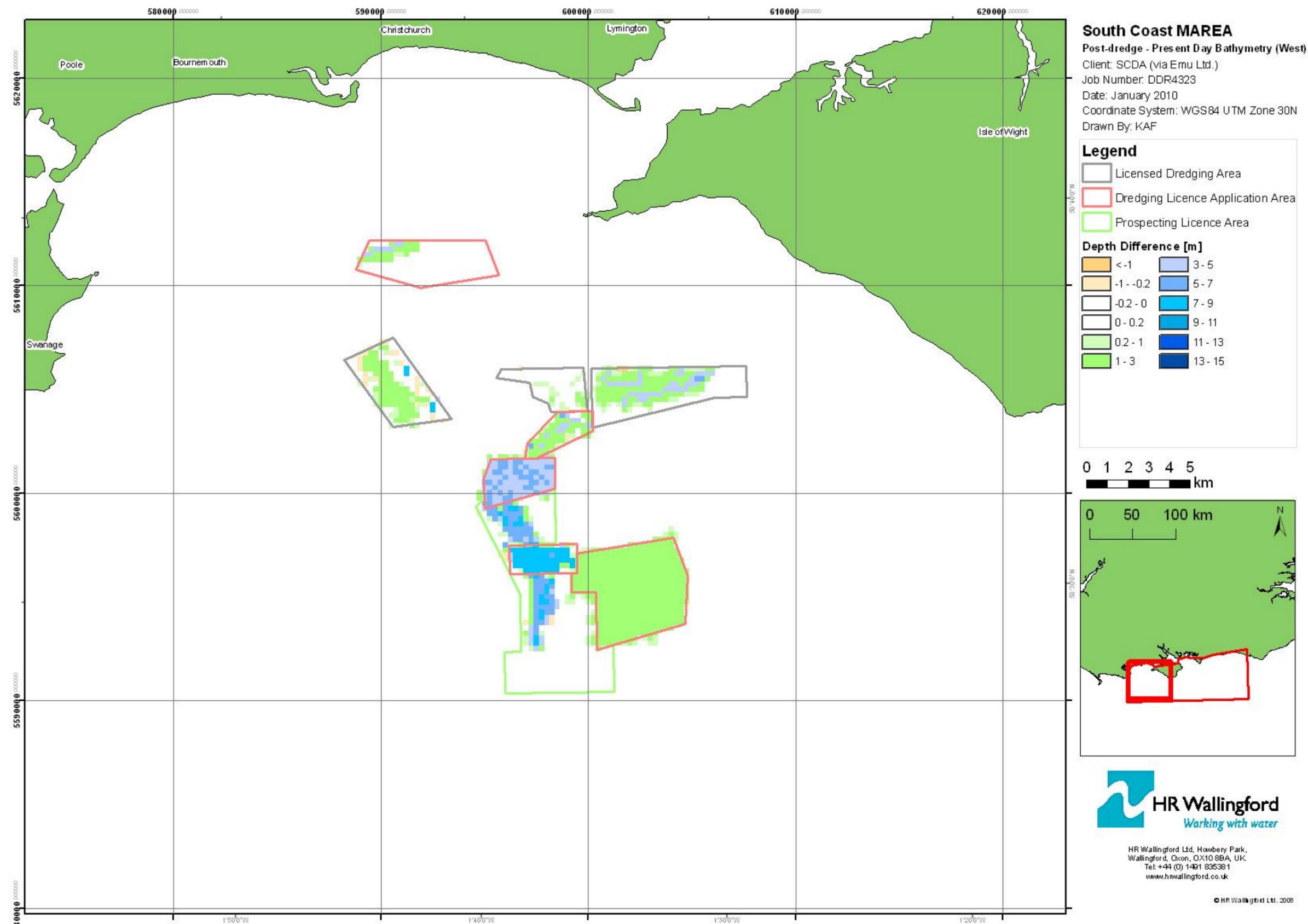


Figure 5 Plans for future depth changes in aggregate dredging areas west of the Isle of Wight

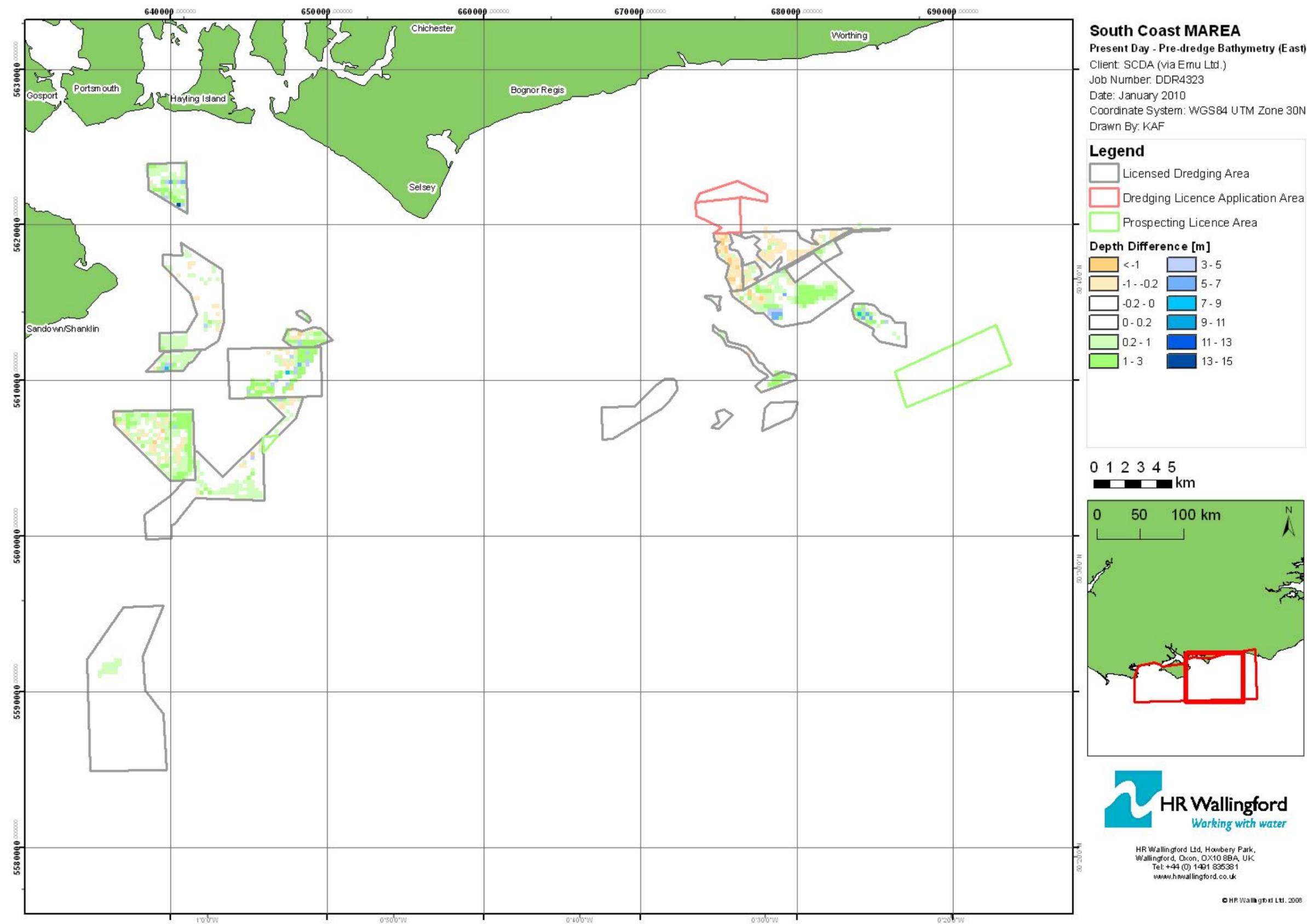


Figure 6 Past depth changes within existing licensed dredging areas east of the Isle of Wight

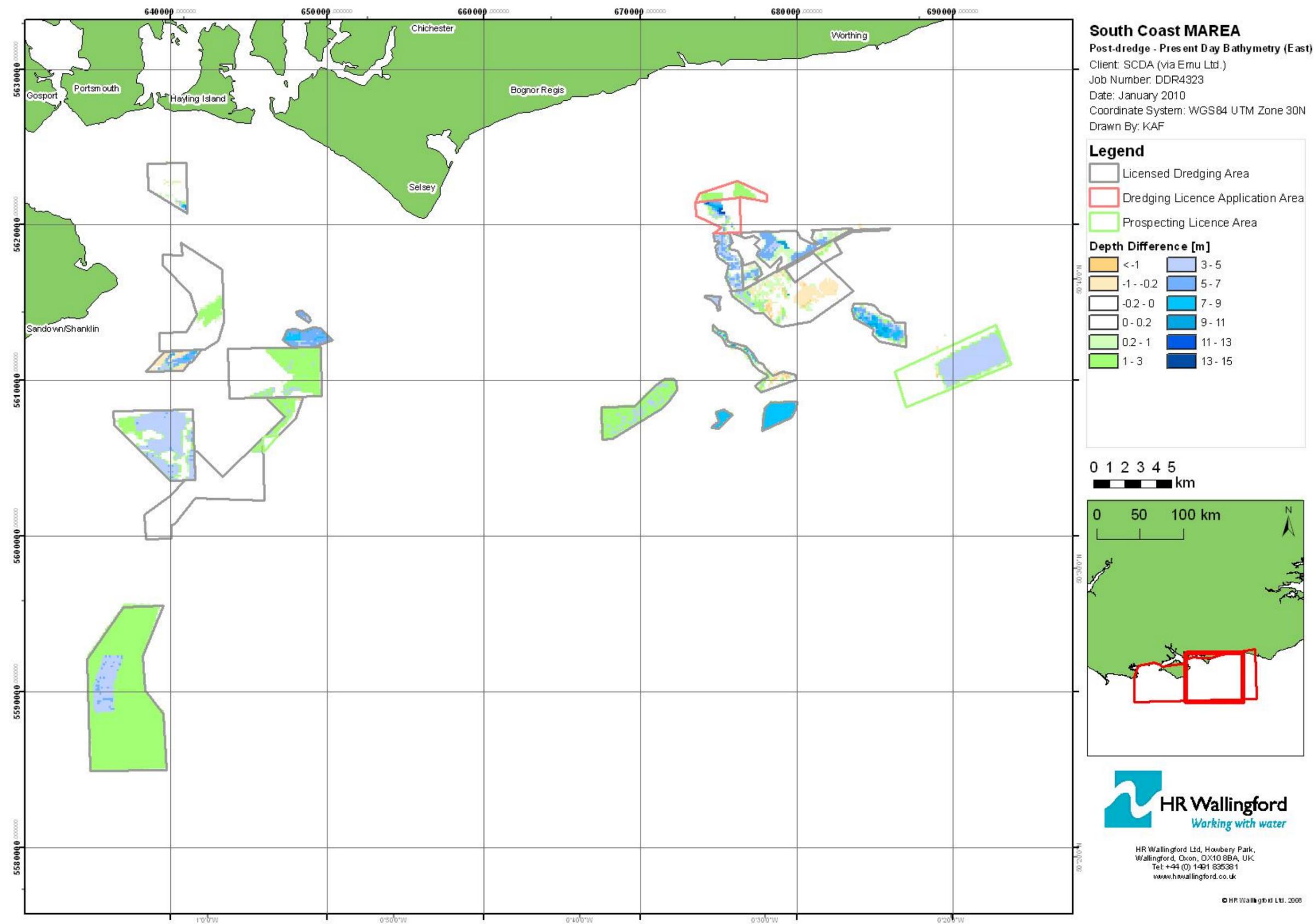


Figure 7 Plans for future depth changes in aggregate dredging areas east of the Isle of Wight

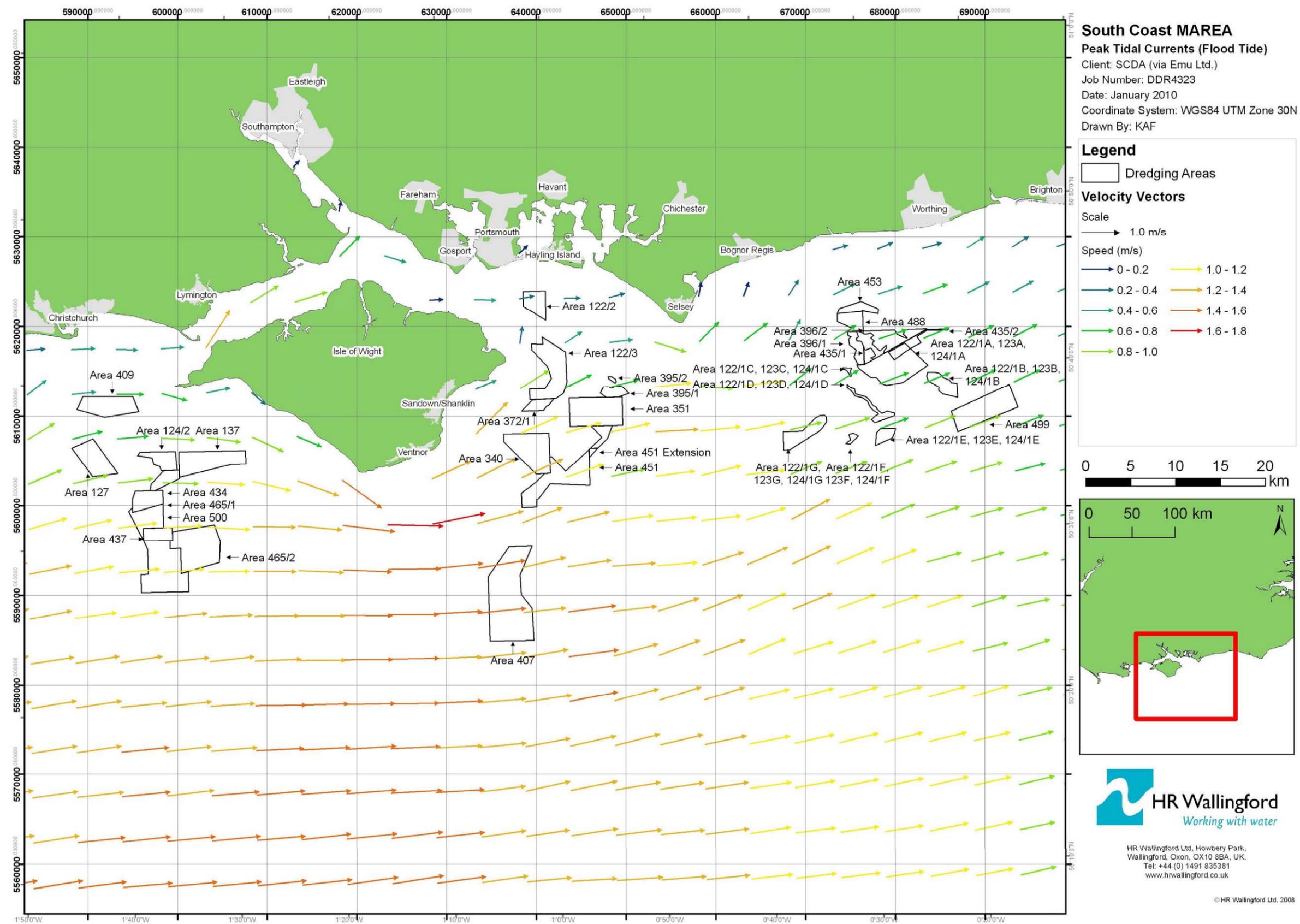


Figure 8 Peak tidal currents – flood tide (present day bathymetry)

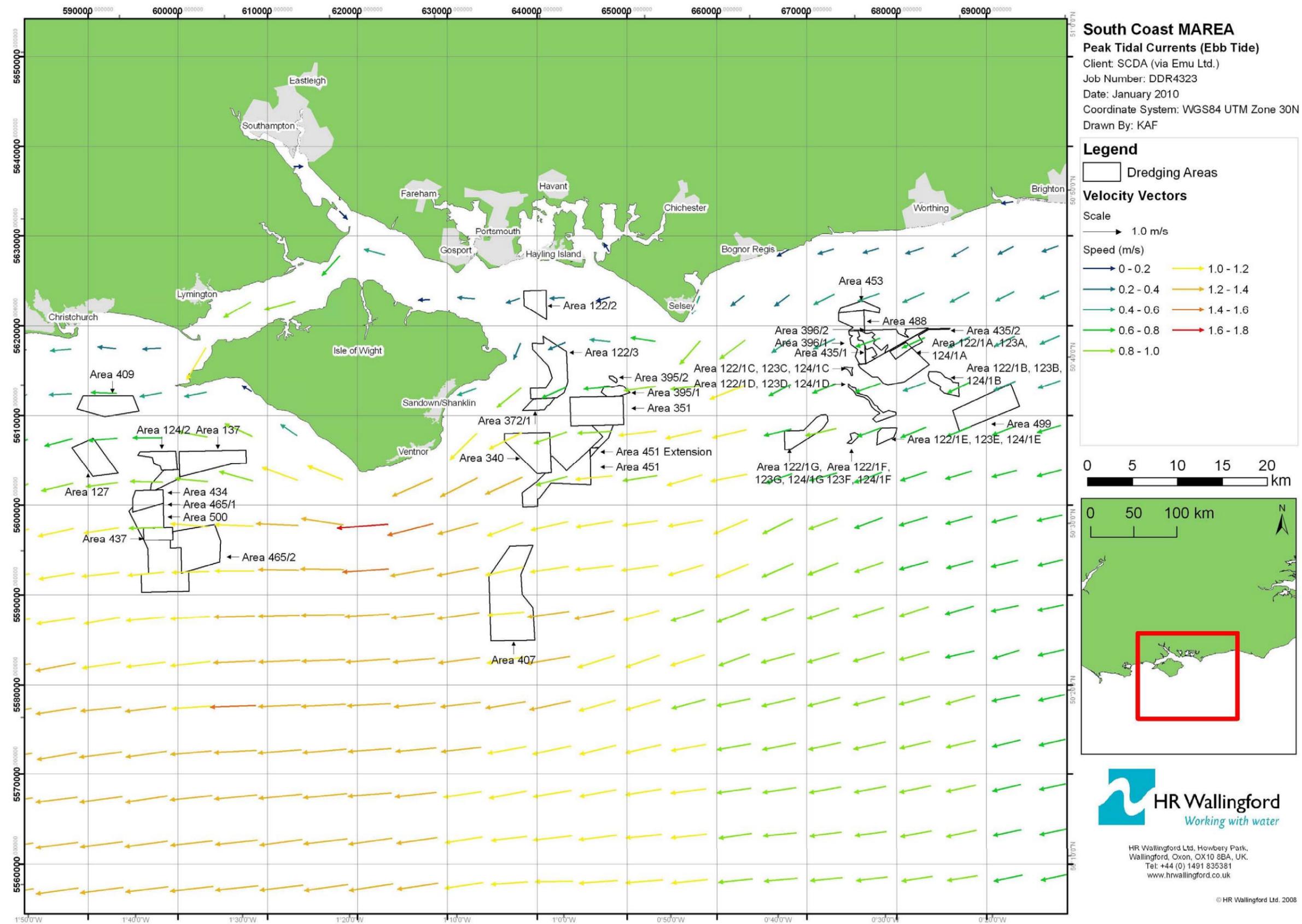


Figure 9 Peak tidal currents – ebb tide (present day bathymetry)

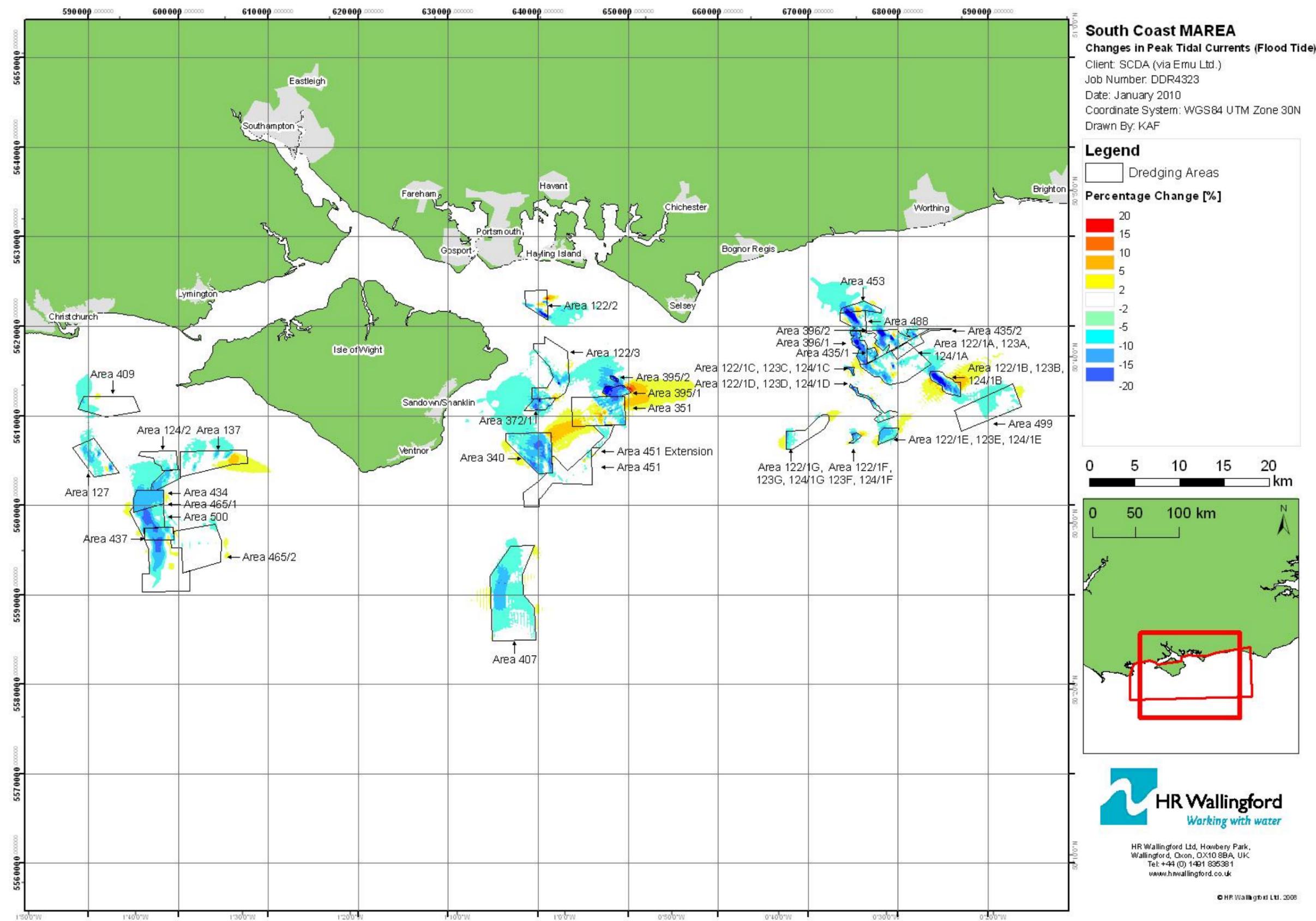


Figure 10 Changes in peak tide currents (post-dredge – pre-dredge) – flood tide

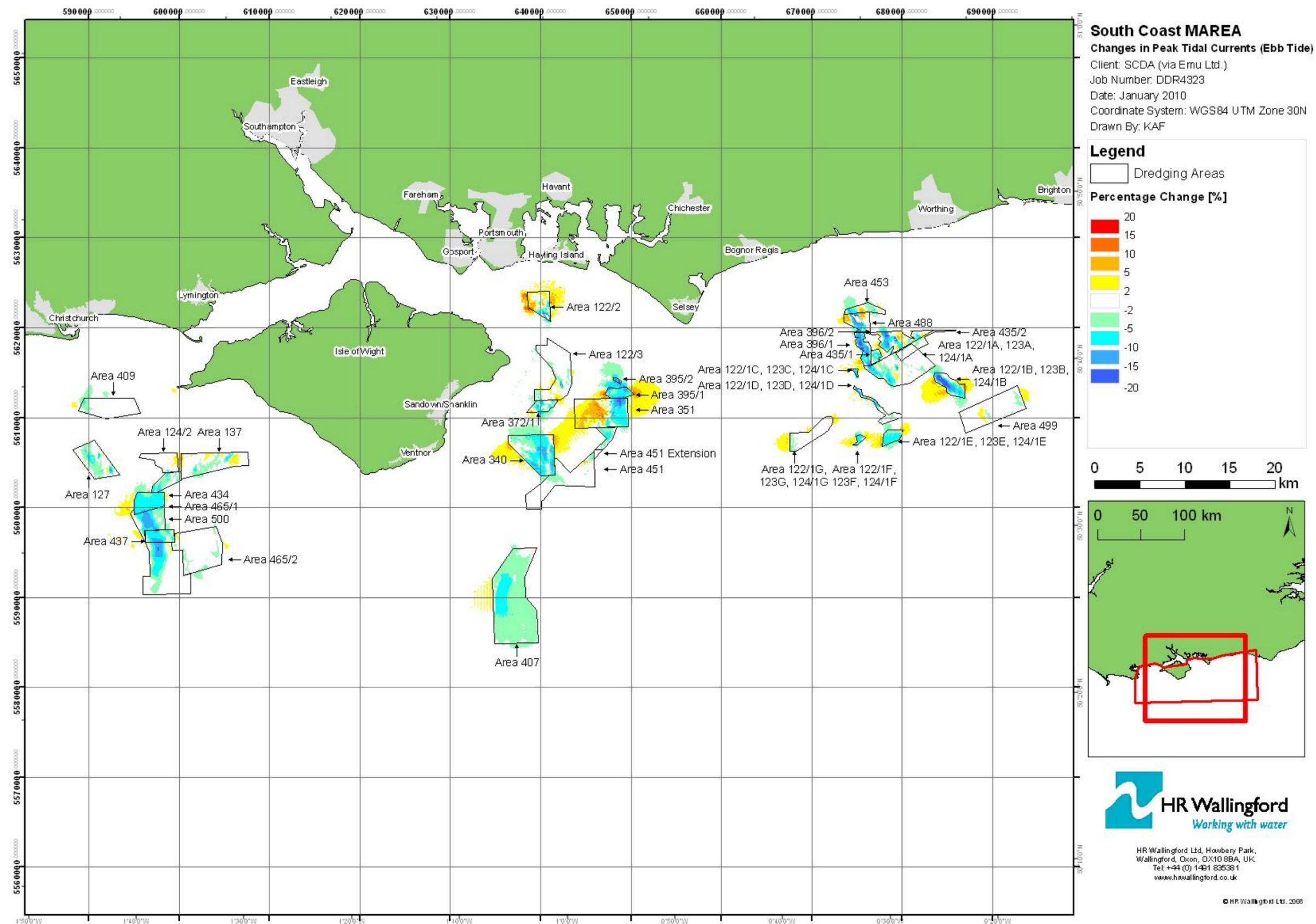


Figure 11 Changes in peak tide currents (post-dredge – pre-dredge) – ebb tide

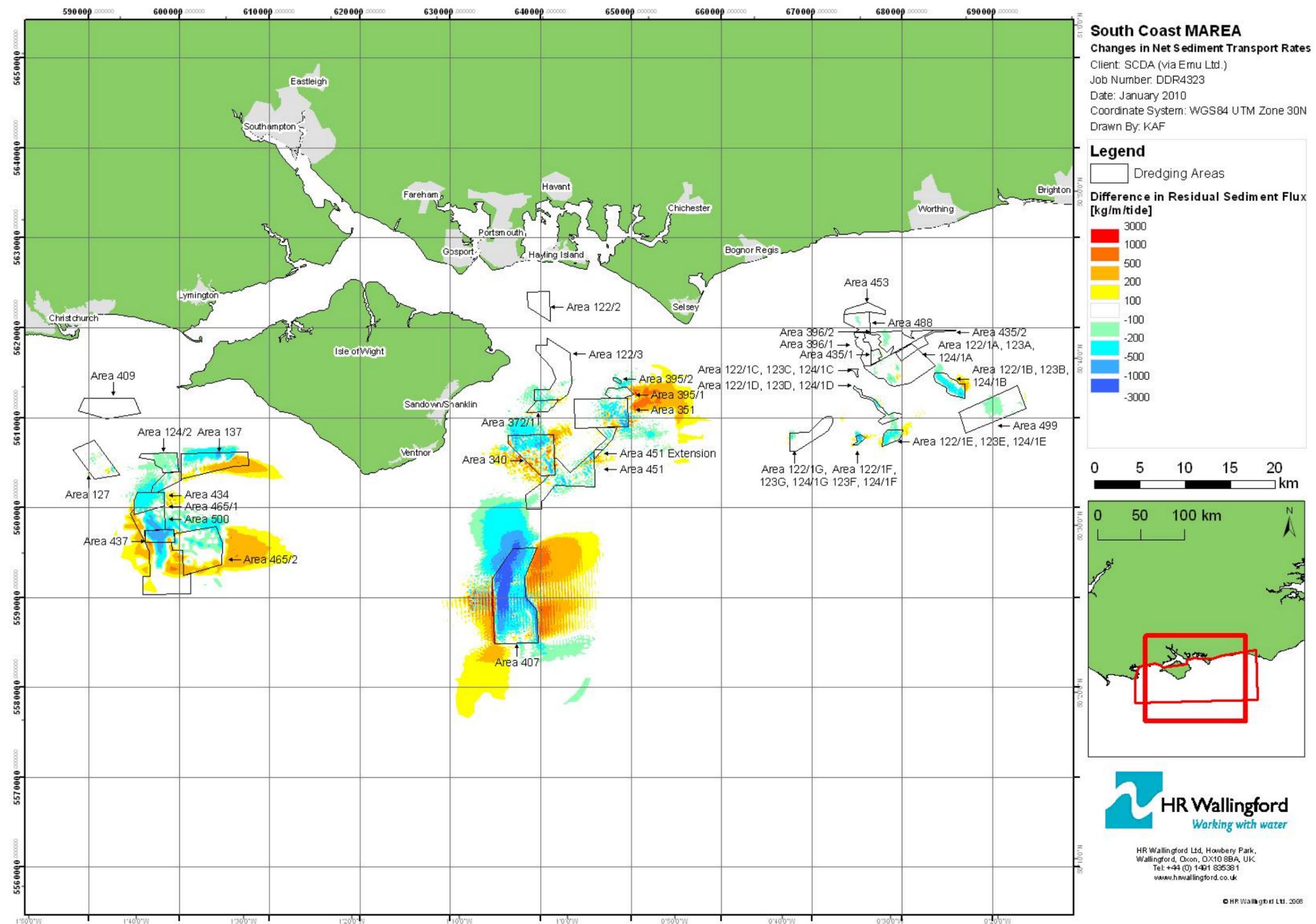


Figure 12 Changes in net sediment transport rates (post-dredge – pre-dredge) (0.3mm sand)

Appendix 1 The TELEMAC Tidal Flow Model

Description of model and main areas of application

TELEMAC-2D is a sophisticated flow model, which was originated by LNH in Paris, for free surface flows. It solves the 2D depth-integrated shallow water equations that are used to model flows in rivers, estuaries and seas. It uses finite element techniques so that very flexible, unstructured triangular grids can be used. It has been developed under a quality assurance system including the application of a standard set of validation tests.

The model can simulate depth integrated tidal flows in estuaries and seas including the presence of drying banks. It can also simulate flows in rivers including turbulence structures resulting from flow obstructions and transcritical flows.

The advantage of using finite elements lies primarily in the possibility of using a very flexible grid. This is superior to using an orthogonal curvilinear grid as the user has far more complete control over grid refinement with a finite element system.

The applications of TELEMAC have included studies of tidal flows, storm surges, floods in rivers, dam break simulations, cooling water dispersion and infill of navigation channels.

Theoretical background and solution methods

TELEMAC solves the shallow water equations on an unstructured finite element grid (usually with triangular elements). The various variables (bed elevation, water depth, free surface level, and the u and v velocity components) are defined at the nodes (vertices of triangles) and linear variation of the water and bed elevation and of the velocity within the triangles is assumed.

When the model is used a time-step is chosen and the computation is advanced for the required number of time-steps. There is no particular limit on the time-step for a stable computation but it is best to ensure that the Courant number based on propagation speed is less than about 10. It is found that if the solution is nearly steady then few computational iterations are required at each step to achieve the required level of accuracy, which in TELEMAC is computed according to the actual divergence from the accurate solution. The computation at each time-step is split into two stages, an advective step and a propagation-diffusion step.

The advective step

The advective step is computed using characteristics or stream-wise upwind Petrov-Galerkin. The characteristic step makes it possible for the code to handle such problems as flow over a bump giving rise to locally supercritical flow and eddies shedding behind flow obstructions.

The propagation/diffusion step

The finite element method used is based on a Galerkin variational formulation. The resulting equations for the nodal values at each time-step are solved using an iterative method based on pre-conditioned conjugate gradient (PCG) methods so that large problems are solved efficiently. Several PCG solvers are coded and a selection is available to the user. The complete matrix is not assembled. Instead an element by element method is used so that most of the operations are carried out on the element matrices; this is computationally more efficient, both in speed of execution and in memory requirements. Rather than using Gauss quadrature exact analytical formulae

are used for the computation of matrices. Symbolic software was used to draw up the formulae used. The software makes it possible to carry out a second iteration of the solution at each time-step in order to represent the non-linear terms in a time centred way, otherwise these terms are treated explicitly.

Boundary conditions

Boundary conditions are applied at solid boundaries where a "zero normal flow" and either a slip or non-slip boundary condition are applied. At open boundaries a selection of possibilities can be invoked depending on whether the flow is subcritical or supercritical or whether a wave absorbing boundary using a Riemann invariant is needed. A water discharge along a boundary segment can also be applied and the software distributes the flow along the segment chosen. This facility is valuable when running models of river reaches and the discharge in a cross section may be known rather than the velocity at each point in the cross-section.

Grid selection

The model can be run with a Cartesian grid for modelling rivers, estuaries and small areas of sea, with the possibility to apply a uniform Coriolis parameter, or on a spherical grid for larger areas of sea in which case the Coriolis parameter is computed from the latitude at each node. The effect of a wind blowing on the water surface and causing a set-up or wind induced current or of an atmospheric pressure variation causing an inverted barometer effect can be included, as can a k-epsilon model of turbulence if required.

Friction

The bed friction can be specified via a Chezy, Strickler or linear coefficient, or a Nikuradse roughness length. A variable friction coefficient over the model area is a possibility. Sidewall friction can also be included if wanted. Viscosity can be imposed as a given eddy viscosity value or a k-epsilon model can be used if needed.

Tracer calculation

TELEMAC-2D includes also the capability to simulate the transport of a tracer substance. The tracer is again computed using an advective step followed by a propagation/diffusion step. Tracer boundary conditions can be applied at model inflow boundaries. The tracer calculation has been used in order to simulate cooling water dispersion and mud transport. Sources of water and/or of tracer can be specified in terms of the discharge required and the x and y co-ordinates of the location.

INPUTS

TELEMAC requires as input a finite element grid of triangles covering the area to be modelled. Bathymetric data from which the bed elevation at each node can be computed is also required covering the area. A file of keyword values is used to steer the computation (supplies bed roughness, time-step, duration of run etc).

Methods of inputting the data

The finite element grid may be provided by a standard FE grid generator such as IDEAS or SIMAIL. The software STBTEL (part of the TELEMAC suite) is used to read the output file from the grid generation software. The bathymetry is input using a digitising tablet and the SINUSX software is used to capture the bathymetry data. The data is stored in a form to be read into the TELEMAC system and depths interpolated to the model nodes.

Methods of checking and amending input data

SINUSX is a powerful interactive graphical software that can be used to check and amend the input data. Bathymetric curves can be duplicated, deleted, smoothed, moved etc.

Time to set up/calibrate/run/amend model

This depends on the form in which the data is supplied. Typically 1-2 days to digitise the chart data and 1-2 days to create the finite element grid. Boundary conditions may take a day to prepare. A run may take 1 to 5 hours to run a tide (for a 2000 cell model). The duration of the calibration process is hard to generalise as it depends entirely on particular circumstances.

OUTPUTS

Output parameters

The user can select from a range of output parameters including u and v velocity, u and v discharge, water level, bed level, water depth, tracer concentration and Froude number.

Output files

The TELEMAC output is contained in a single binary file which can be input to the graphics post-processor RUBENS. A listing file contains reflection of the input keywords and information on time-step reached, number of iterations to convergence etc. This file can be used to monitor the progress of a run.

Output plots

Results from the TELEMAC system are processed using the interactive graphics system RUBENS. This is a powerful and friendly environment in which figures can be produced interactively. By pointing and clicking time history plots, cross sections, vector plots and contour plots of any parameter at any position can be produced. Parameters other than those input can be calculated in RUBENS and plotted.

GENERAL

Interaction and compatibility of the model with other models

The main modules apart from TELEMAC-2D itself (the 2D flow model code) are SINUSX and RUBENS (described above).

The TELEMAC suite includes a bed load transport model (TSEF) and a suspended load model (SUBIEF). Also a wave model ARTEMIS that solves the mild slope equation.

The TELEMAC modelling suite also includes a quasi-3D random walk model for pollution transport modelling and a detailed water quality model with many water quality parameters including dissolved oxygen balance and particulates.

Quality Assurance

The software has been developed under the quality assurance procedures required by the French Electricity Industry. This has included the production of an extensive dossier of validation tests.

Validation

Validation tests on TELEMAC include:

- Simulation of eddies produced behind bridge piers. This test case includes the ability of the model to produce an unsteady solution from steady boundary conditions (von Karman vortex street).
- Drying on a beach.
- Simulation of the tides on the continental shelf including the Bay of Biscay. This model has been closely compared with the observed tides at coastal sites.
- Flow over a step in the bed with critical flow and a hydraulic jump. This solution is compared with the analytically known solution to this problem.

Appendix 2 The SANDFLOW Sediment Transport Model

Description of model and main areas of application

SANDFLOW-2D is the sand transport modelling module of the TIDEWAY-2D system. SANDFLOW-2D uses the flows calculated by TIDEFLOW-2D to study the transport, deposition and erosion of non-cohesive (sandy) sediment and thereby identify areas of potential siltation and erosion. SANDFLOW-2D has also been adapted to use the flows produced by the TELEMAC flow model for sand transport calculation.

Theoretical background and solution methods

The sediments under consideration here are very fine and fine sands ($d_{50} \sim 0.06$ to 0.25 mm) which mainly move in suspension. The model can also be used to identify trends in the case of medium sand ($d_{50} \sim 0.25$ to 0.5 mm). If the sediment contains a high proportion of clay or silt particle sizes less than 0.06 mm, it would be more appropriate to use the MUDFLOW-2D (TIDEWAY) or SUBIEF (TELEMAC-2D) models.

The main factors controlling sand transport are:

- advection by currents
- settlement under gravity
- turbulent diffusion in all directions (but only the vertical component is of significance under most circumstances)
- exchange of sediment between the flow and the bed

The study of sand transport generally is very difficult but more so in the case of estuaries or coastal areas. This is because the water movements are continually changing, with the rise and fall of the tide, and there is usually a wide range of sediments on the bed and areas without mobile sediment, leading to unsaturated loads in the water.

Method

Although sand transport in estuaries is really an unsteady, 3D problem, it has been shown by HR Wallingford that it can be dealt with using a 2D, depth-averaged model provided special provision is made to account for the vertical profile effects of the sediment concentration. Under these circumstances the depth-averaged, suspended solids concentration $c(x, y, t)$ satisfies the conservation of mass equation.

$$\frac{\partial}{\partial t}(dc) + \alpha \left[\frac{\partial}{\partial x}(duc) + \frac{\partial}{\partial y}(dvc) \right] = \frac{\partial}{\partial s} \left(dD_s \frac{\partial c}{\partial s} \right) + \frac{\partial}{\partial n} \left(dD_n \frac{\partial c}{\partial n} \right) + S \quad (1)$$

where

- (u,v) = depth-averaged components of velocity (m/s)
- D_s = longitudinal (shear flow) dispersion coefficient (m^2/s)
- D_n = lateral (turbulent) diffusivity (m^2/s)
- (x,y) = Cartesian co-ordinates in horizontal plane (m)
- (s,n) = natural co-ordinates (parallel with and normal to mean flow) (m)
- t = time (sec)
- d = water depth (m)
- S = erosion from or deposition on the bed ($kg/m^2/s$)
- α = advection factor to recover the true sediment flux from the product of

depth-averaged quantities

Advection factor (α)

This is introduced to compensate for the omission of the vertical profile in the sediment flux terms.

$$\alpha = T/qcd \quad (2)$$

where

$$\begin{aligned} T &= \int_0^d q'c'dz \text{ is the sand transport (kg/m width/s)} \\ q &= \text{the depth-averaged water speed } (u^2 + v^2)^{1/2} \end{aligned}$$

and q',c' are the full three-dimensional velocity and concentration variables.

Since the highest concentrations occur near the bed it follows that $\alpha \leq 1$. Typical values of α can be obtained by evaluating equation (2) for sand transport profile observations or from the integration of theoretical solutions for suspended solids profiles. However, in practice, it is usually acceptable to take $\alpha = 1$ on the grounds that the external and internal sources of mobile sediment are not well enough known to justify a more precise formulation.

Bed exchange relations

The simplest formulation of the bed exchange relation is

$$S = \beta_s \omega_s (c_s - c) \quad (3)$$

where

- c_s is the depth-averaged concentration when the flow is saturated with sediment (kg/m^3)
- ω_s is the representative settling velocity (m/s)
- β_s is a profile factor to compensate for integrating out the vertical profile of suspended sediment ie to correct for higher sediment concentrations near the bed.

Deposition or erosion takes place depending on whether the instantaneous sediment load (c) exceeds or is less than the saturated value (c_s). Pick up of sediment from the bed is prevented if there is no sediment available on the bed. A shortage of material on the bed is reflected in a low concentration of suspended solids being advected away by the flow.

Typical values of β_s could be obtained from actual observations of sediment profiles or from theoretical considerations. However, HR Wallingford has derived an analytical expression for this so that bed exchanges are performed automatically. This involves simplifying the vertical diffusivity relation and a profile mixing factor is introduced to enable the user to increase or decrease the effective mixing during calibration of the model.

Sediment transport relation

The evaluation of bed exchanges requires a depth-averaged sediment concentration (c_s). Sandflow-2D obtains this from a sediment transport relation specified by the user.

Three sand transport relations are supplied in the package (Ackers-White, van Rijn and a simple power law) and since the source code is provided other relationships can be added by the user if preferred.

The choice of sand transport relation needs care. It should be borne in mind that most relationships found in the literature are based on river or channel data where sediments are more narrowly graded than in estuaries. Also there is normally a small proportion of cohesive material in estuary sediments and this can alter the transport properties. If possible, sand fluxes should be measured at the study site, and if such data is available it may be best to use it to obtain the best-fit power law relation for the site.

Diffusion

The dispersion (D_s) and diffusion (D_n) coefficients are not well defined. When viewed in close enough detail the whole motion appears advective; but when viewed on a coarser grid the smallest motions appear diffusive. Thus selection of the appropriate diffusion or dispersion coefficients depends on the grid size of the model - one model will treat as advection what a coarser grid model will treat as diffusion or dispersion.

Fortunately, the solutions to the equation are not normally sensitive to D_s and D_n . As a first approximation, $D_n = Bdu$, where d and u are representative depths and velocities. It has been found that B is usually in the range 0.01 (for fairly uniform depths and smooth beds) to 0.1 (for irregular geometry and/or rougher beds).

D_s is automatically calculated by the program for each model cell depending on the local depth and velocity to give more diffusion in the direction of flow. The overall scale of D_s can be changed using the relative dispersion parameter (in keyword DIFFUSION). This normally has the value unity but it can be adjusted upwards or downwards during calibration to get agreement between the model results and any dispersion observations that may be available.

Numerical model

A simple, explicit, upstream finite difference technique is used to solve the advection - diffusion equation. Flux corrections are not considered to be necessary because the background concentrations of suspended sand are normally fairly uniform throughout the model in contrast to POLLFLOW-2D applications that have one or two point sources and correspondingly steeper concentration gradients.

The use of an explicit method introduces a stability constraint on the computing time step (Δt).

$$\Delta t < \Delta s / (\text{maximum flow velocity})$$

where Δs is the grid size (TIDEWAY) or separation between nodes (TELEMAC-2D) in metres.

Generally, this does not pose any problems in practice because the allowable Δt is usually much larger than the TIDEFLOW-2D time step and there is only a single equation to solve in the process model compared to three in TIDEFLOW-2D. Under these circumstances an explicit method is preferred because it enables the user to understand the code more easily and to modify the treatment of the physics of the processes being simulated. Note that where TELEMAC-2D is being used the values of Δs will vary and so the minimum value of Δs is the most important in terms of stability.

The treatment of the dispersion (D_s) and diffusion (D_n) terms introduces another stability constraint.

$$\Delta t < \Delta s^2 / 4 D_{\max}$$

where D_{\max} is the maximum of D_s and D_n .

This constraint is normally weaker than the advective stability limit but the user should be aware that a high value of diffusivity can lead to an instability. In the event of problems the possible violation of both limits should be checked.

Application of the model

The application of the model and interpretation of the results requires a good understanding of sand physics. Firstly it is important to choose representative values for the main parameters. Ideally these should be based on laboratory tests of actual sediment samples from the site. It is also important for the modeller to be aware of the limitations of this type of model when applied to real sites.

In addition it should be appreciated that sand transport is not an exact science. Accordingly, whatever model is used, and whatever parameter values are chosen it is essential that results are interpreted correctly. Provided this is done the model will be a valuable engineering tool.

Calibration/validation

Calibration of sediment models is difficult because bed changes are usually too slow or too variable to measure anything significant for comparison. Sometimes historical charts or dredging records may be available but even then it is unlikely that the sources of suspended sediment can be quantified for the relevant period. Sometimes it is possible to get scaling factors for model results in cases where information is available and use these to estimate siltation in the new situation, but in many cases one is forced to use the best available values for the parameters and to demonstrate that the siltation and erosion patterns produced by the model agree with the observed state of the estuary or coastal region being studied.

Some evidence to support the physical realism of the model is given by the following results of simulation of sand transport in a flume and of observations from the Thames estuary.

The computer model results were compared with the results of a laboratory experiment performed in a flume with a length of 30m, a width of 0.5m and a depth of 0.7m. The discharge was measured by a circular weir. The mean flow depth was 0.25m and the mean flow velocity was 0.67 m/s. The bed material had a $d_{50} = 230\mu\text{m}$ and a $d_{90} = 320\mu\text{m}$. The median diameter of the particles in suspension was estimated to be about 200 μm , resulting in a representative fall velocity of 0.022 m/s (water temperatures 9°C). The stream bed was covered with bed forms having a length of about 0.1m and a height of about 0.015m. Small Pitot tubes were used to determine the vertical distribution of flow velocity. Water samples were collected simultaneously by means of a siphon method at four locations to determine the spatial distribution of the sand concentrations. At each location (profile) five samples were collected at a height of about 0.015, 0.025, 0.05 and 0.22m above the average bed level and these were integrated to give the suspended load transport. The HR SANDFLOW-2D model was run for the same

conditions assuming the overall shear velocity was 0.0477 m/s and the results in Figure 1 shows that the model could be calibrated if suitable data is available.

The model was compared with some flume data to test its response to a change in the sediment load. It was shown that the model simulation could be calibrated by adjusting the settling velocity and vertical diffusivity parameters. This procedure is justified for practical applications because in nature these parameters are not well defined. For example, there is no unique settling velocity because the suspended load would contain a range of sediment sizes and the true nature of the vertical diffusivity is not yet fully understood.

The basic physics of the model was then checked against real field data from Foulness in the Thames Estuary. There was a wide range of sediment sizes in the data but the model was only used to simulate individual fractions. The saturation concentrations in the model were calculated using a cubic velocity relation derived from the observed sand fluxes.

Results from the model simulation of the 75 to 100 μm sand fraction are shown in Figures 2 and 3 plotted at half hourly intervals with a sequence number showing the progression through the tide. The model has a similar hysteresis effect to the observations on both stages of the tide. The systematic underestimation of concentrations during the ebb is probably due to a different availability of sediment sizes not allowed for in the simplified model. Nevertheless the demonstration confirms the general validity of the model in a natural situation.

An example of the agreement achieved during validation, between the SANDFLOW-2D model results and observed sediment distribution is shown in Figure 4. Note in particular the agreement between the areas of potential erosion predicted by the model and areas of rock bed, and also the areas of potential deposition and areas of sand bed observed.

INPUTS

Input data required

SANDFLOW-2D requires as input the elevation and flow results from a TIDEFLOW-2D run or a TELEMAC-2D run, together with information describing the initial distribution of sand on the bed. A boundary data file is also required to specify sediment concentrations at the model edges with respect to time. Other parameters required include the typical size of sand and its basic properties such as settling velocity and threshold stress for initiation of motion.

Methods of inputting the data

Data is input to SANDFLOW-2D using ASCII data and steering files and unformatted direct access results files from TIDEFLOW-2D or the TELEMAC-2D equivalent. The steering files are set up using the context sensitive editor included in the user interface.

User interface

A keyword driven interface controls all aspects of using SANDFLOW-2D from setting-up a model through to analysis of the results. The interface includes file management functions, graphical presentation of results and utilities for results analysis and file format conversion.

OUTPUTS

Output parameters

SANDFLOW-2D calculates concentrations of suspended sediment and distributions of erosion and deposition are stored at user selected intervals during the run. SANDFLOW-2D calculates suspended sediment concentration, erosion and deposition throughout the model area for each time step through the tide.

Output files

Each run of the SANDFLOW-2D model generates three output files. Two of these files contain the suspended concentrations and bed deposits. The third output file; the List File contains run information.

Output plots

The results from the SANDFLOW-2D may be represented using report quality-graphics utilities included in the TIDEWAY-2D system or, where TELEMAC-2D has been deployed, the RUBENS visualisation system. Contour plots of suspended concentrations and/or bed deposits at user selected times and concentration-time and deposit-time plots at selected locations can be produced.

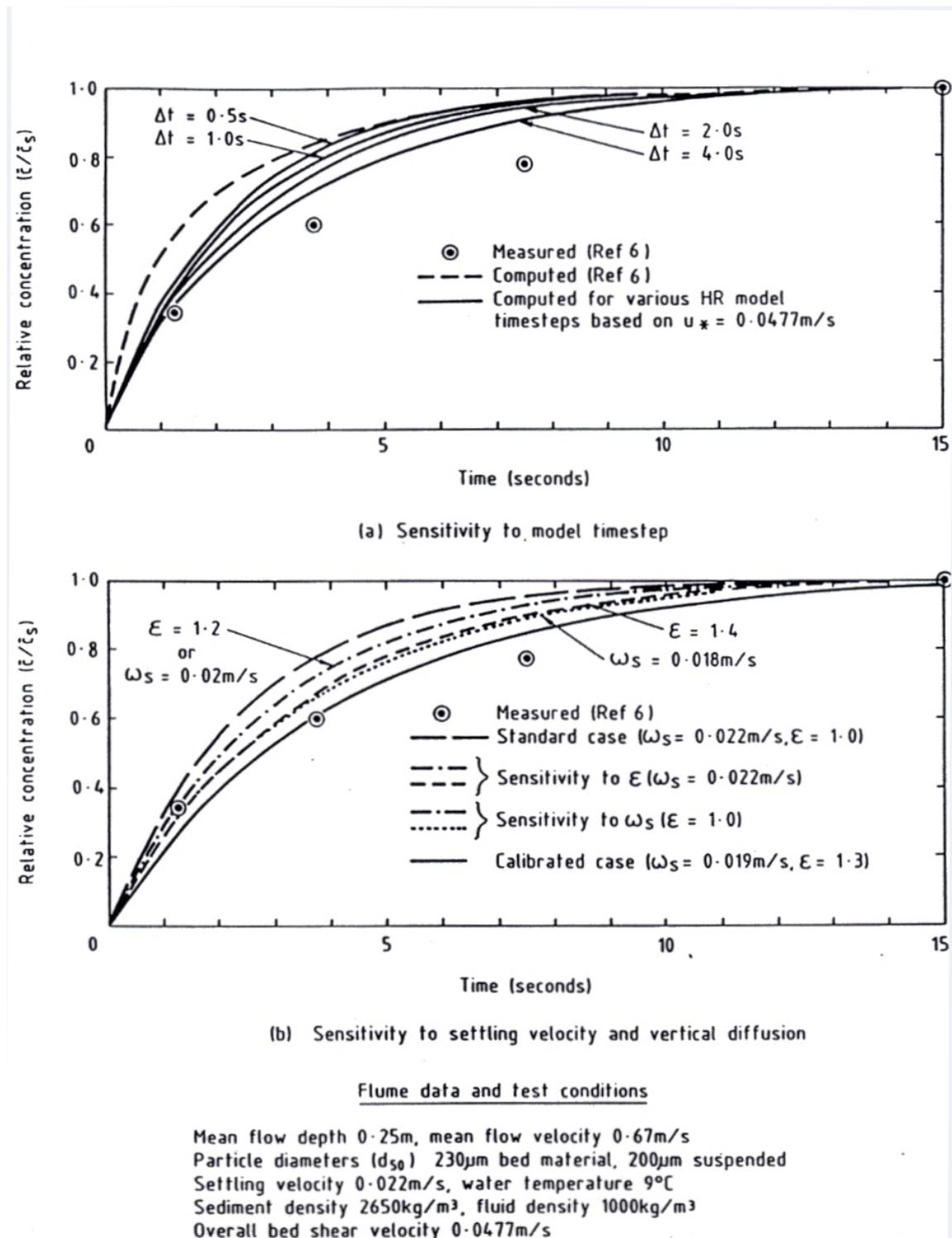


Figure 1 Computed and measured evolutions of sediment load

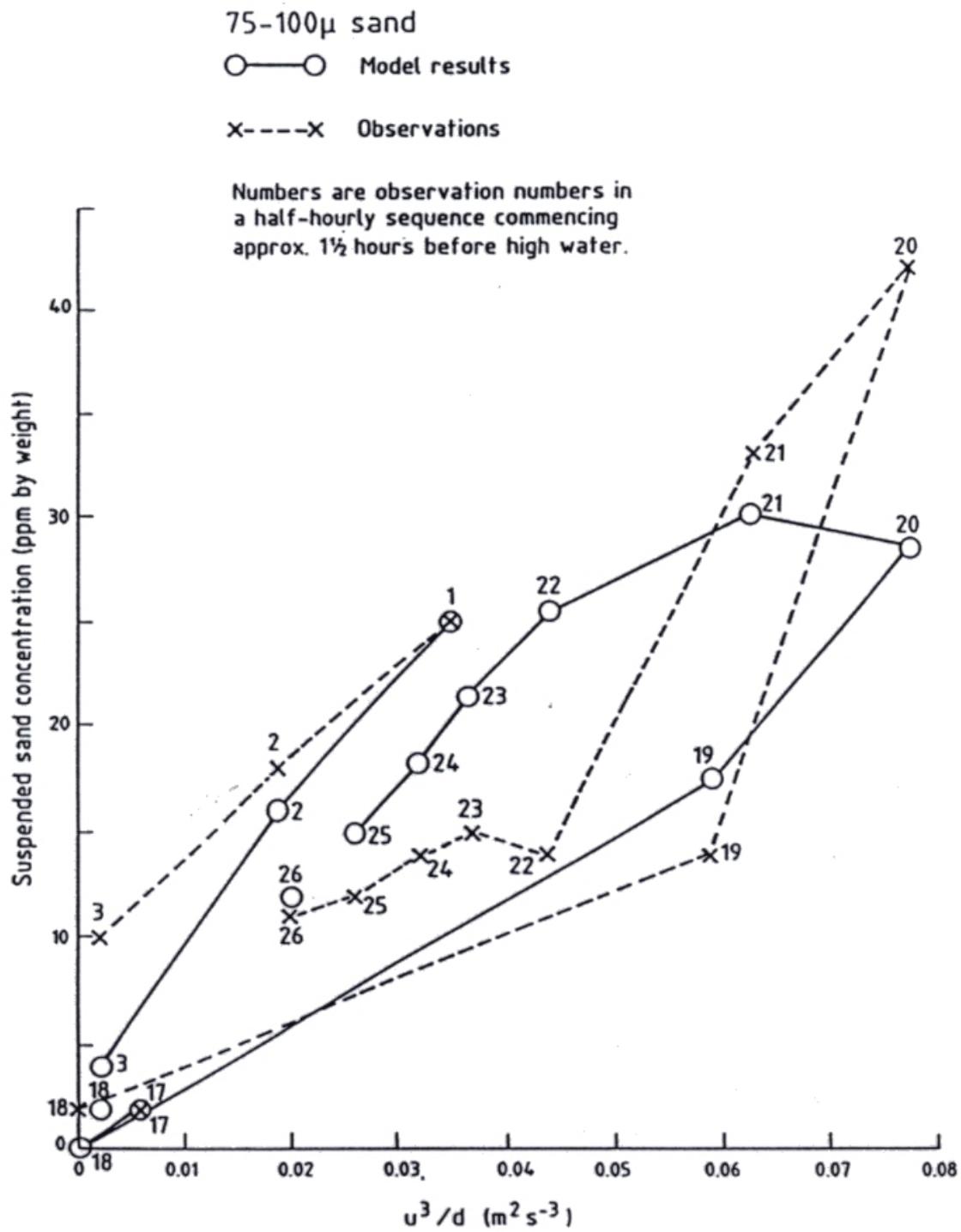


Figure 2 Simulation of Foulness position 1 flood tide

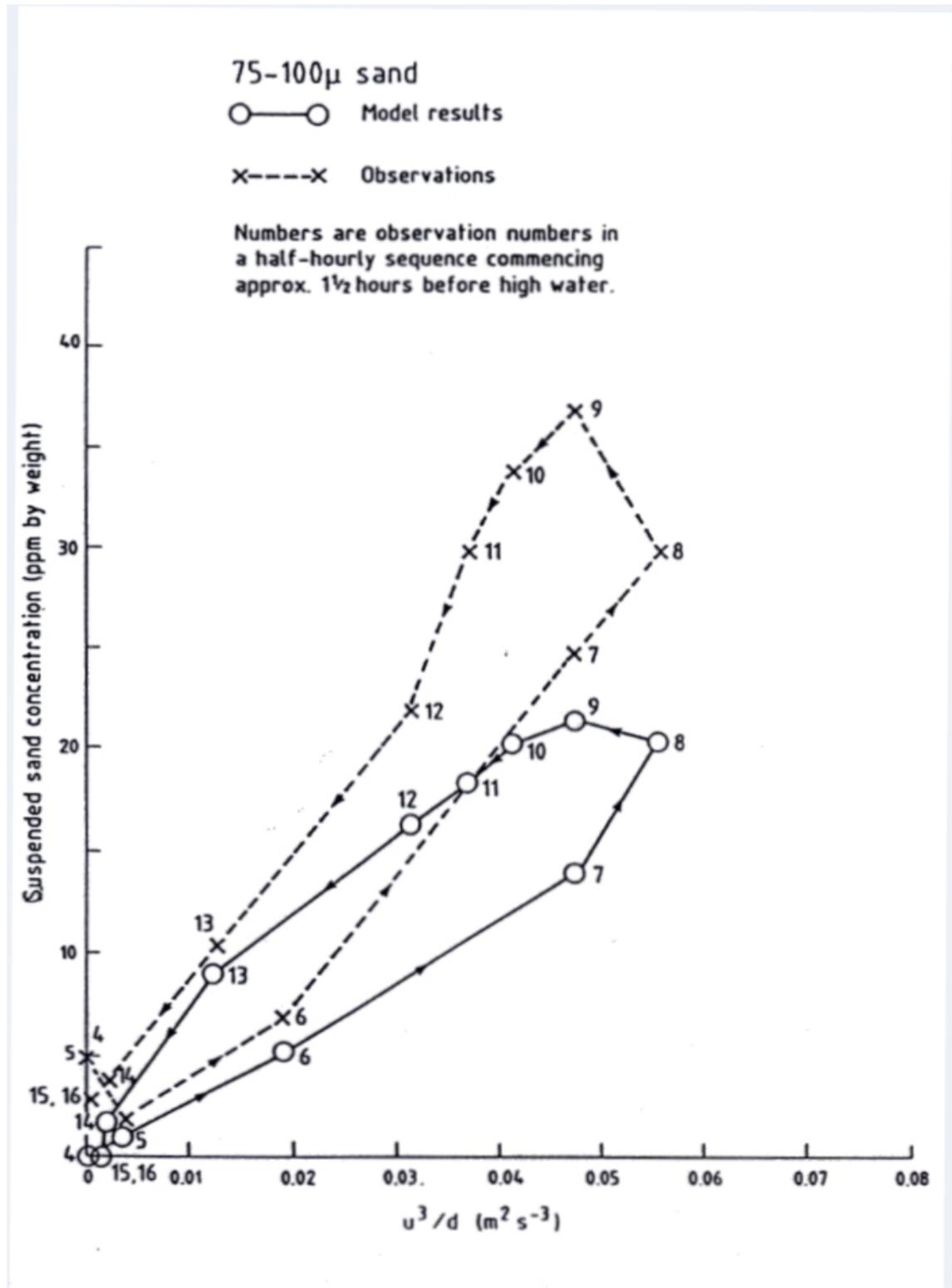


Figure 3 Simulation of Foulness position 1 ebb tide

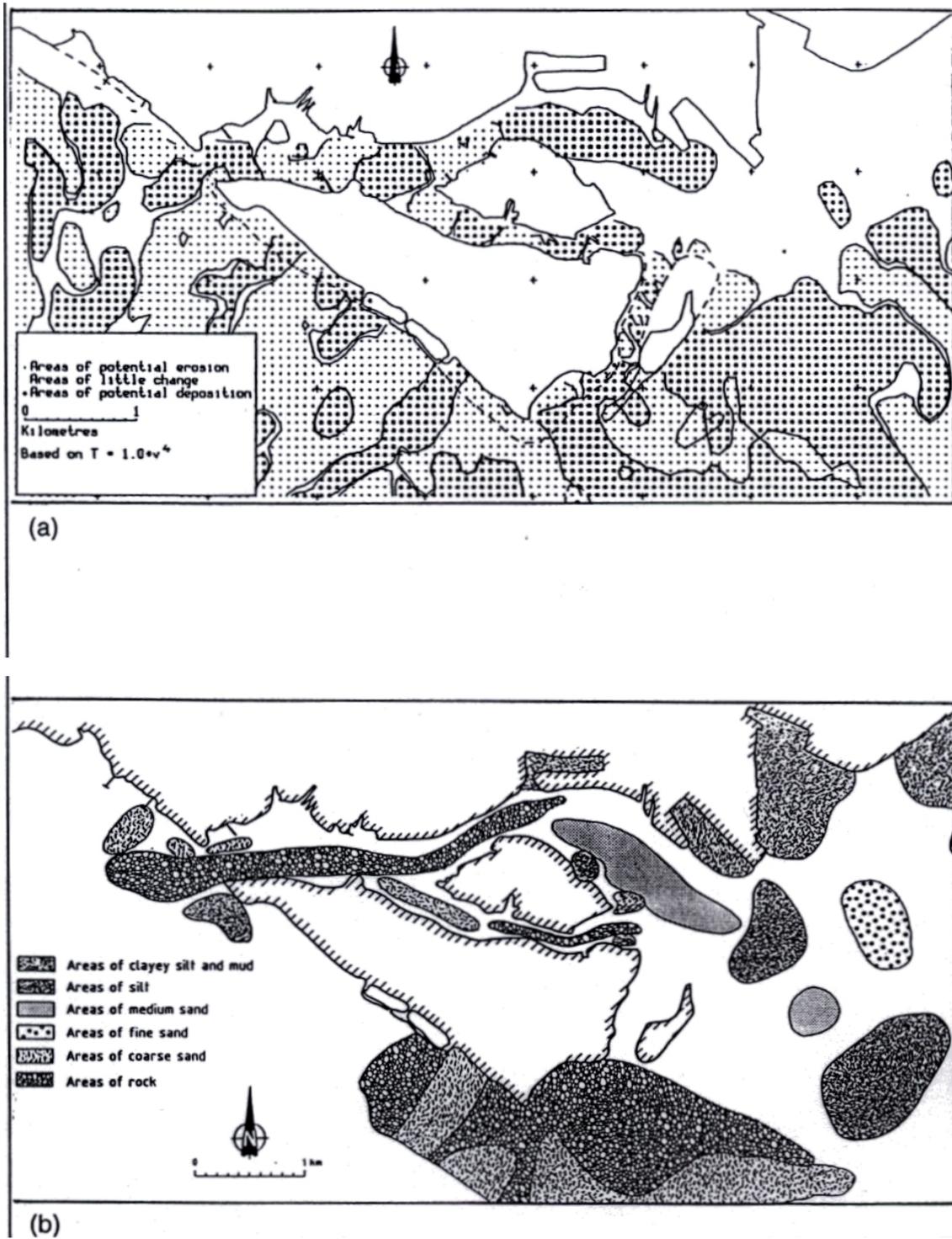


Figure 4 Comparison of sand erosion/deposition areas in model (a) with observed sediment distribution (b)

Appendix 3 Bathymetry Data Sources

Company	AREA	PRE-DREDGE	PRESENT	POST-DREDGE
CMX	Area A 137	See file 137 OSGB pre.xls	See file 137 OSGB now.xls	See file See file 137 OSGB post.xls
CMX	Nab 340	Baseline data set, 1993 (see file 340 OSGB pre.xls)	340 WGS84_UTM now.xls (received from EMU)	Lower current bathy in resource zones (see GIS files: 340 OSGB res zone boundary_region) by 5m.
CMX	St Catherines 407	Seazone and 0.5m shallower in dredged area (see GIS files 407 LL WGS dredge boundary)	Seazone	Seazone minus 6m in resource area(see GIS files: 407 LL WGS res zone boundary) Revised bathy: northern left edge was removed, areas to the right and bottom lowered by 5m, those to the left lowered by 3m, see "3207_001.pdf"
CMX	Inner Owers 453	Seazone	Seazone	Seazone minus 2m in resource area (files: 453 OSGB res zone boundary-region)
UMD	Area 122/1A 123A 124/1A	Area 122_1 Zone A&D Pre Dredge Bathy OSGB36.txt	Area 122_1 Zone A&D Present Day Bathy OSGB36.txt	Area 122_1 Postdredge Bathy OSGB36.txt
HAML	Area 122/1B 123B 124/1B	124-1 AreaB Predredge contours_polyline.shp	124-1 AreaB contours_polyline.shp (2004)	124-1 Area B Post dredging.txt
HAML	Area 122/1C 123C 124/1C	124-1 Area C pre-dredge grid.txt	124-1 Area C Current.txt (2003)	Area C Postdredge grid 2 final -ve.txt
UMD	Area 122/1D 123D 124/1D	Area 122_1 Zone A&D Pre Dredge Bathy OSGB36.txt	Area 122_1 Zone A&D Present Day Bathy OSGB36.txt	Area 122_1 Postdredge Bathy OSGB36.txt
CMX	Area 122/1E 123E 124/1E	Seazone	Seazone	123 EFG LL WGS res zone boundary_region.shp 8m lowering
CMX	Area 122/1F 123F 124/1F	Seazone	Seazone	123 EFG LL WGS res zone boundary_region.shp 8m lowering
CMX	Area 122/1G 123G 124/1G	Seazone	Seazone	123 EFG LL WGS res zone boundary_region.shp 3m lowering
UMD	Area 122/2	Area 122_2 Pre_Present_Post Bathy Data.txt	Area 122_2 Pre_Present_Post Bathy Data.txt	Area 122_2 Pre_Present_Post Bathy Data.txt Revised bathy: new bathy provided by UMD "Area 122_2 Post Dredge Bathy UTM30N v2 0809.txt"

Company	AREA	PRE-DREDGE	PRESENT	POST-DREDGE
UMD	Area 122/3	Area 122_3 Southern Boundary Trimmed Pre Dredge Bathy OSGB36.txt	Area 122_3 Southern Boundary Trimmed Present Day Bathy OSGB36.txt	Area 122_3 Post Dredge Bathy UTM30N.txt Revised bathy: new bathy provided by UMD "Area 122_3 Post Dredging Bathy OSGB36 v2 0809.txt"
UMD	Area 395	Area 395 Pre Dredge Bathy OSGB36.txt	Area 395 Present Day Bathy UTM30N.xyz	Area 395 Post Dredge Bathy UTM30N v2 0309.txt
UMD	Area 396	Area 396 Pre Dredge Bathy OSGB36.xls	Areas 396 & 435 Present Day Bathy UTM30N.txt	Area 396 Post Dredge Bathy UTM30N.txt
UMD	Area 488	Area 488 Pre Dredge & Present Day Bathy UTM30N.txt	Area 488 Pre Dredge & Present Day Bathy UTM30N.txt	Area 488 Post Dredge Bathy UTM30N.txt
UMD	Area 435	Predredge_bathy_restored_polyline.shp	Areas 396 & 435 Present Day Bathy UTM30N.txt (2004)	435_PostDredge_bathy.txt
UMD	Area 434 437 500	Areas 434_437_500 Pre Dredge & Present Day Bathy UTM30N.txt	Areas 434_437_500 Pre Dredge & Present Day Bathy UTM30N.txt	Areas 434_437_500 Post Dredge Bathy UTM30N.txt
WGL	Area 451	451_UTMzone30.xyz	451A.txt and 451B.txt and Seazone for the rest of the areas	3.0m lowering – see word document for coordinates of area to be dredged (worst case used)
NLF/VDL	Area 351	351_91.xyz	351.txt and Seazone for the rest of the areas	3.0m lowering – see word document for coordinates of area to be dredged
HAML	Area 124/2	124-2_predreddebathy_restored_polyline.shp	124-2Depth2000_poly1m_polyline.shp (2000)	124-2_Giveback_bathy_final.txt
HAML	Area 372	372&213_predredge_contour_polyline.shp	finalHanson_372_1.txt (2006)	372-1 Postdredge bathy.txt
HAML	Area 409	Seazone	Seazone	Isopach_Depth_trim_contour_polyline.shp
HAML	Area 499	Seazone (hasn't been dredged yet)	Seazone	4m lowering – see 499 Indicative Application area 0608_region.shp
HAML	Area 127	127_bathymetry_restored_polyline.shp	2000surveydepth_contour_polyline.shp (2000)	127 postdredge bathy.txt
VDL/ Britannia	Area 477	Seazone	Seazone	See document ("HR Future Bathy.wmf ") provided for coordinates of resource area and 2m lowering

Company	AREA	PRE-DREDGE	PRESENT	POST-DREDGE
HAML	Area 465/1	Seazone	Seazone	Seazone + see pdf document ("application_chart_south_sept 08.pdf") provided for coordinates of resource area and email outlining 5m lowering
HAML	Area 465/2	Seazone	Seazone	Seazone + see document ("application_chart_south_sept 08.pdf") provided for coordinates of resource area and email outlining 2m lowering

